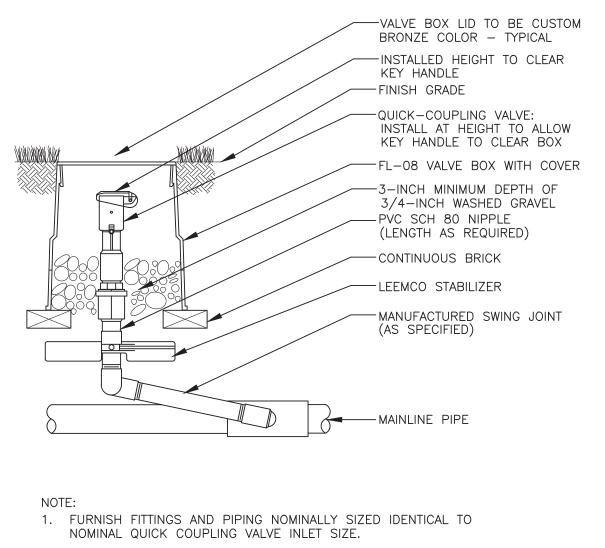
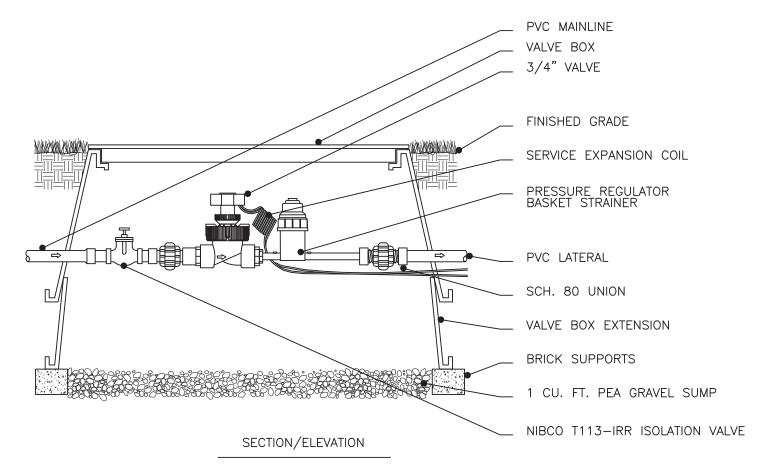


1 ISOLATION VALVE WITH BOX



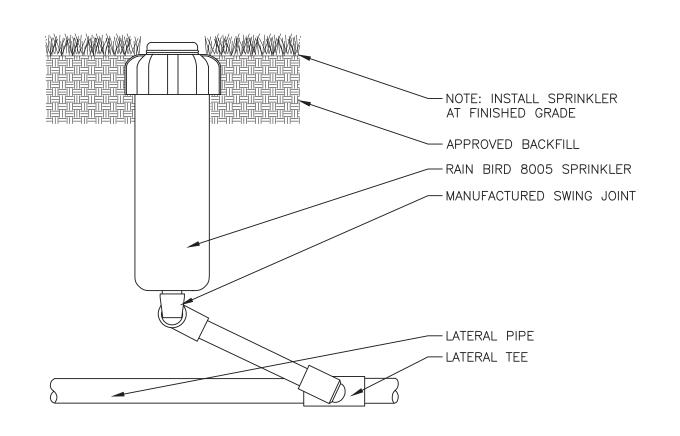
QUICK-COUPLING VALVE
WITH STABILIZER

SCALE: NONE



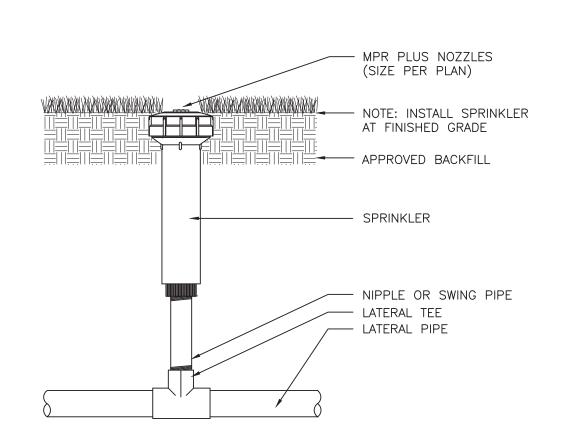
REMOTE CONTROL VALVE,
PRESSURE REGULATOR & FILTER

SCALE: NONE



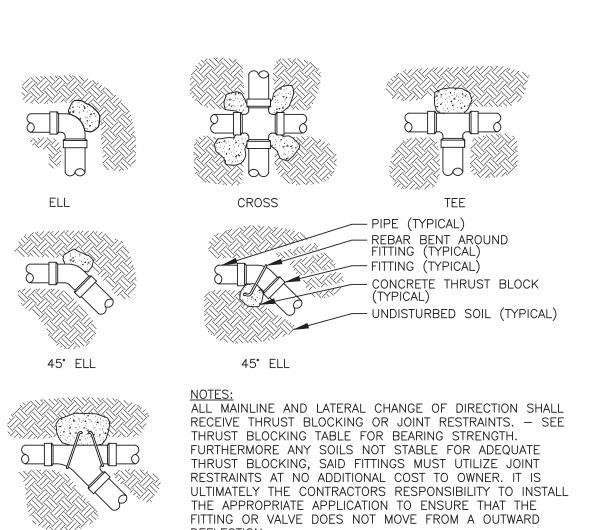
4 TURF ROTOR

SCALE: NONE



5 FIXED-SPRAY SPRINKLER

SCALE: NONE



THRUST BLOCKING SIDE THRUST ON CURVES STEP 1. MULTIPLY THE PRESSURE LEVEL DESIRED AN OUTWARD PRESSURE EXISTS ON ALL DEFLECTIONS FOR TESTING BY THE APPROPRIATE VALUE SHOWN IN FROM A STRAIGHT LINE. GOOD SOIL, PROPERLY THE FOLLOWING TABLE: TAMPED, IS SUFFICIENT TO HOLD SIDE THRUST -UNLESS SOIL CONDITIONS ARE UNSTABLE. IN THAT PIPE DEAD END 90° 45° 22.5° CASE, TO ANCHOR AGAINST THIS SIDE THRUST, THE SIZE OR TEE ELBOW ELBOW ELBOW BLOCKING SHOULD BE PLACED AGAINST THE PIPE ON EACH SIDE OF THE COUPLING. DO NOT THRUST BLOCK 1.5" 2.94 4.16 2.25 1.15 THE COUPLING ITSELF. 4.56 6.45 3.50 1.78 PIPE SIZE SIDE THRUST 9.40 5.10 2.60 2.5" 6.65 INCHES POUNDS PER DEGREE 3" 9.80 13.9 7.51 3.82 1.5" 3.5" 12.8 18.1 9.81 4.99 7.9 16.2 23.0 12.4 6.31 2.5" 11.6 5" 24.7 35.0 18.9 9.63 17.1 6" 34.8 49.2 26.7 13.6 22.4 28.3 BASED ON POUNDS PER PSI WORKING PRESSURE. 43.1 60.8 STEP 2. DETERMINE THE BEARING STRENGTH OF THE SOIL FROM THE TABLE BELOW: BASED ON SIDE THRUST PER 100 LBS/SQUARE INCH BEARING STRENGTH OF SOILS PRESSURE PER DEGREE OF DEFLECTION SOILS AND SAFE BEARING LOADS LBS. SQ. FT. NOTE: MULTIPLY SIDE THRUST POUNDS BY DEGREES SOUND SHALE OF DEFLECTION TIMES POUNDS OF PRESSURE DIVIDED BY 100 TO OBTAIN TOTAL SIDE THRUST IN POUNDS. CEMENTED GRAVEL AND SAND-DIFFICULT TO PICK COARSE AND FINE COMPACT SAND MEDIUM CLAY -2,000 1,000 CAN BE SPADED SOFT CLAY MUCK STEP 3. DIVIDE THE TOTAL THRUST OBTAINED IN STEP 1 BY THE BEARING STRENGTH OF THE SOIL: THIS GIVES THE SQUARE FEET OF AREA NEEDED.

SLEEVE MARKER -(#4 REBAR STAKE END PAINTED "BLUE AND BLUE TRACER WIRE WRAPPED AROUND BAR) SCH. 40 IRRIGATION PVC PIPE -(SEE PLAN FOR SIZE) CURB LINE 4 4 4 4 4 3" SCH. 80 PVC -(WIRE SLEEVE - GRAY IN COLOR) AS PER CODE -SCH. 40 PVC PIPE SLEEVE (SIZED TWO TRENCH -PIPE SIZÉS LARGER (BACKFILLED THAN SLEEVED AND COMPACTED) IRRIGATION PIPE NOTED ALL ROAD CROSSINGS ARE SUBJECT TO CITY FINAL LOCATION AND APPROVAL ON PLAN) BEFORE INSTALLATION - TYPICAL BACKFILL MATERIAL W/ SAND 2" ABOVE PIPE SLEEVE WARNING TAPE 12" BELOW GRADE OVER PIPE SLEEVING DETAIL

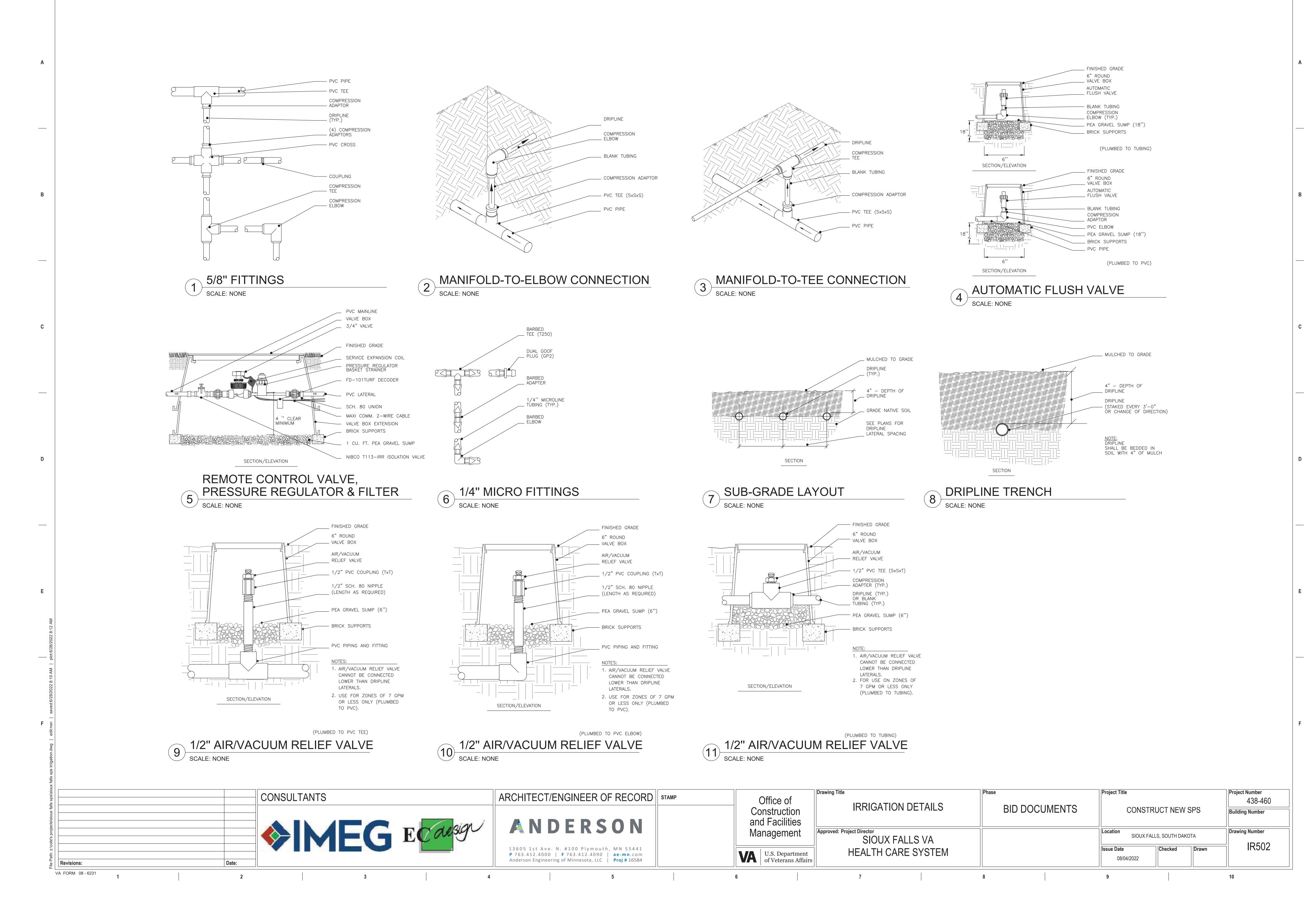
SCALE: NONE

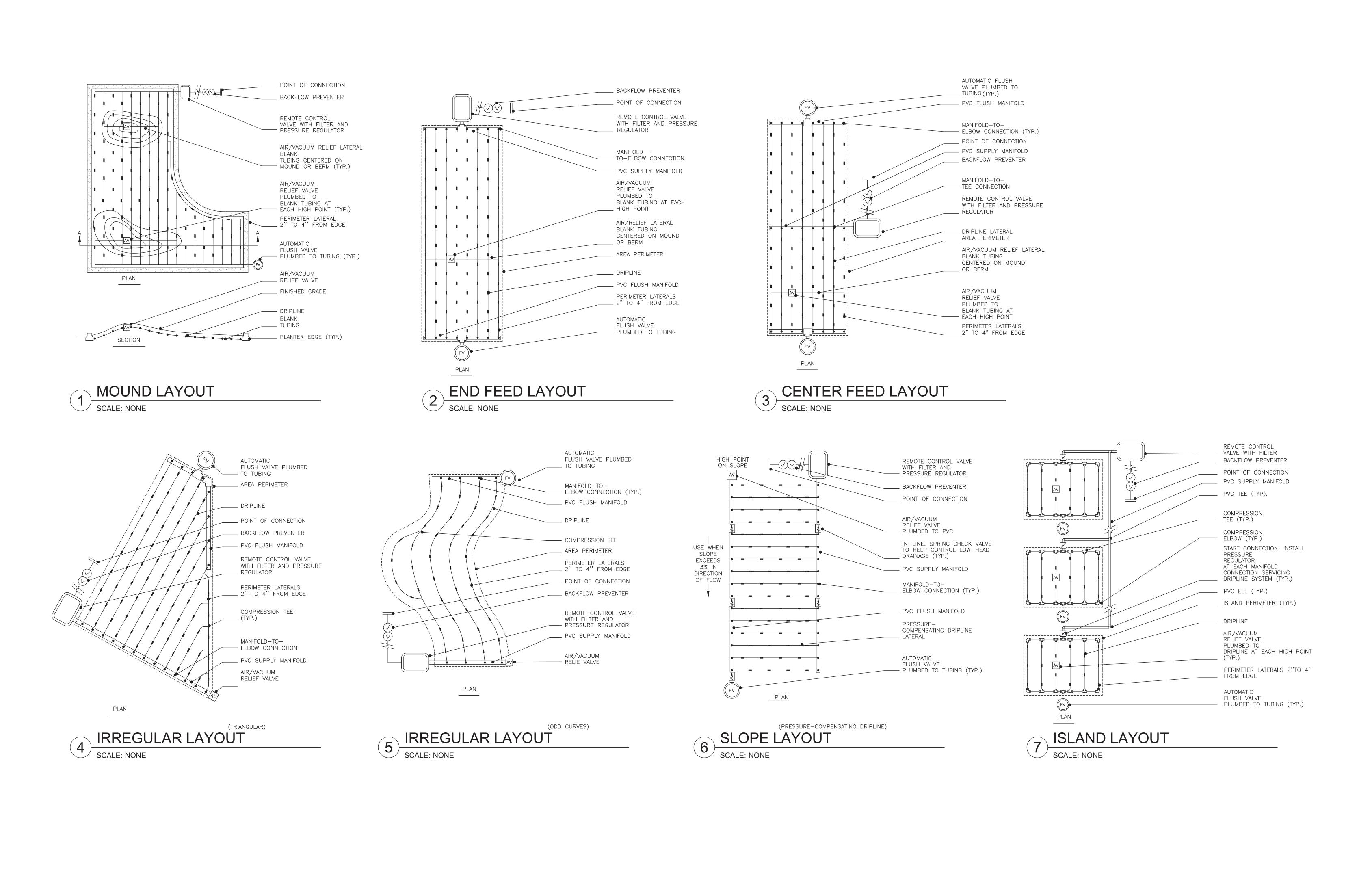
6 THRUST BLOCK DETAIL
SCALE: NONE

Drawing Title Project Title Project Number CONSULTANTS ARCHITECT/ENGINEER OF RECORD | STAMP Office of 438-460 IRRIGATION DETAILS BID DOCUMENTS CONSTRUCT NEW SPS Construction **Building Number** and Facilities EG Ecaesign ANDERSON Management **Approved: Project Director** Drawing Number SIOUX FALLS, SOUTH DAKOTA SIOUX FALLS VA IR501 Checked 13605 1st Ave. N. #100 Plymouth, MN 55441 HEALTH CARE SYSTEM U.S. Department of Veterans Affairs P 763.412.4000 | F 763.412.4090 | ae-mn.com 08/04/2022 Anderson Engineering of Minnesota, LLC | Proj # 16584 Revisions: VA FORM 08 - 6231

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Drawing Title

Approved: Project Director

IRRIGATION DETAILS

SIOUX FALLS VA

HEALTH CARE SYSTEM

Office of

Construction

and Facilities

Management

U.S. Department of Veterans Affairs

Project Title

08/04/2022

CONSTRUCT NEW SPS

SIOUX FALLS, SOUTH DAKOTA

Checked

BID DOCUMENTS

Project Number

Building Number

Drawing Number

438-460

IR503

BARBED MICRO VALVE

ARCHITECT/ENGINEER OF RECORD | STAMP

ANDERSON

13605 1st Ave. N. #100 Plymouth, MN 55441

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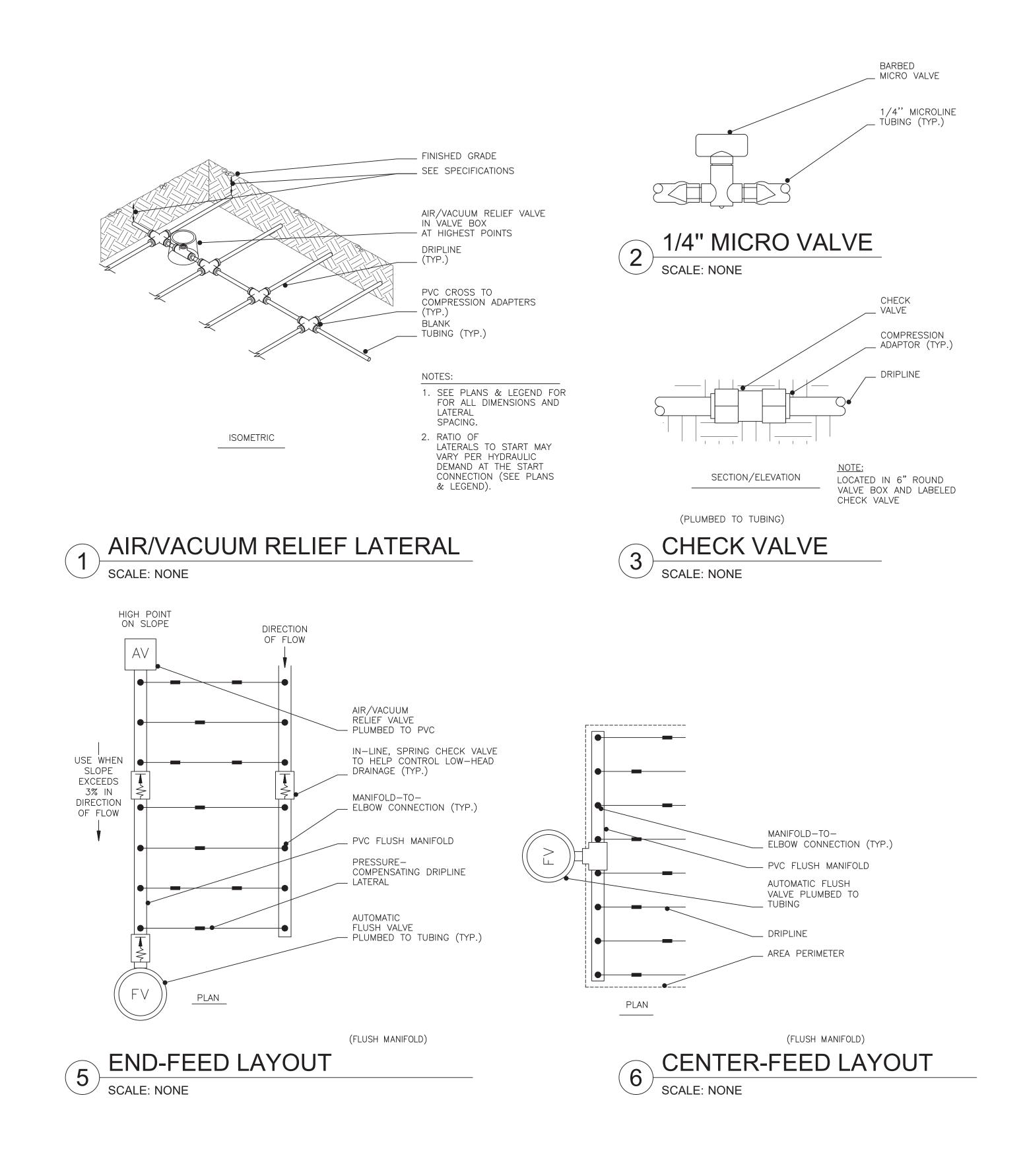
Anderson Engineering of Minnesota, LLC | Proj # 16584

Revisions:

VA FORM 08 - 6231

CONSULTANTS

EG Ecaesign



NOTE: ALL PIPING TO BE LOCATED OUTSIDE OF TREE ROOT BALL PLACE 1st RING
18" FROM TREE TRUNK -PLACE 2ND RING 36" FROM TREE TREE WATERING —— BASIN 48" FROM TREE TEMPORARY TREE EMITTER LAYOUT FOR ESTABLISHMENT

SCALE: NONE

Project Title Drawing Title Project Number CONSULTANTS ARCHITECT/ENGINEER OF RECORD | STAMP Office of 438-460 IRRIGATION DETAILS **BID DOCUMENTS** CONSTRUCT NEW SPS Construction **Building Number** and Facilities ANDERSON EG Ecaesign Approved: Project Director Management Drawing Number SIOUX FALLS, SOUTH DAKOTA SIOUX FALLS VA IR504 Checked 13605 1st Ave. N. #100 Plymouth, MN 55441 **HEALTH CARE SYSTEM** U.S. Department of Veterans Affairs P 763.412.4000 | F 763.412.4090 | ae-mn.com 08/04/2022 Anderson Engineering of Minnesota, LLC | Proj # 16584

Revisions:

VA FORM 08 - 6231

- 3. The contract structural drawings and specifications represent the finished structure. Unless otherwise indicated, they do not indicate the means or method of construction. The contractor is solely responsible for the protection of the structure during all phases of demolition, construction, and installation.
- 4. The finished structure has been designed for the loading indicated below. It is the responsibility of the contractor(s) and their specialty Engineer(s) to review and use means and methods to adequately address loading on the structure during construction including, but not limited to, wind, snow, seismic, underpinning, material storage, and equipment.
- 5. Cross reference all dimensions and details with architectural and mechanical drawings before commencing any fabrication and/or construction.
- 6. Details and conditions not specifically shown shall be constructed in accordance with details shown for similar conditions and materials.
- 7. Shop drawings prepared by suppliers, sub-contractors, etc. shall be reviewed, coordinated, and signed/stamped by the contractor prior to submitting to the Structural Engineer of Record. The Structural Engineer of Record's review of shop drawings, product data, design calculations, etc., does not relieve the contractor from complying with the contract
- 8. Verify location of all box outs and openings. Opening sizes and locations shown for pipes, ducts, mechanical units, etc. are for general information only and shall be verified with all trades before commencing the work.
- 9. Contractor is solely responsible for protection of the existing building during all phases of construction.
- 10. No structural repairs, corrections, or alterations of work affecting a structural member shall be made without the approval of the Structural Engineer of Record. Design and/or review may be an additional service. 11. Do not scale the drawings.

DESIGN CRITERIA LOADS AND STRESSES:

1. International Building Code (2018) 2. Minimum Design Loads for Buildings and Other Structures (ASCE 7-16).

DESIGN LOADS:

SEISM	IIC DESIGN CRITERIA
Risk Category	IV
Seismic Importance Factor, I _e	1.5
Mapped Spectral Response Acceleration Parameters, S _S and S ₁	$S_S = 0.090 \text{ g}$ $S_1 = 0.035 \text{ g}$
Site Class	"D"
Design Spectral Response Acceleration Parameters, S _{DS} & S _{D1}	$S_{DS} = 0.096 g$ $S_{D1} = 0.056 g$
Seismic Design Category	"A"
Basic Seismic Force-Resisting System	Steel ordinary concentrical brace frame and steel ordinary moment frame
Response Modification Coefficients	Steel ordinary conctrically brace frame R = 3.25 Steel ordinary moment grame R=3.0
Seismic Response Coefficients	$C_S = 0.01$
Design Base Shear	V = C _S * W
Analysis Procedure Used	Equivalent Lateral Force Procedure
WINI	D DESIGN CRITERIA
Ultimate Design Wind Speed (3-sec gust), V _{ult}	120 MPH
Nominal Design Wind Speed (3-sec gust), V _{asd}	93 MPH

J (- J), asa	
Risk Category	IV
Wind Exposure	"C"
Internal Pressure Coefficients	$GC_{pi} = +/-0.18$
ROOF S	SNOW LOAD DATA*
Ground Snow Load, Pg	40 PSF
Snow Exposure Factor, Ce	1.0
Snow Load Importance Factor, I	1.2
Thermal Factor, Ct	Heated C _t = 1.0
Slope Factor, C _s	$C_s = 1.0$
Flat Roof Snow Load, P _f	Heated P _f = 35 PSF + drifting

Flat Roof Snow Load, Pf *See Plan for Unbalanced Snow Loads & Snow Drift Loads

FLOOR LIVE LOADS:

150 PSF Mechanical/Electrical areas 100 PSF Stairs, and exits

CONCRETE: (f'c) at 28 Days

- 3000 PSI Footings
- 3500 PSI Slab on grade [max w/c = 0.45, fly ash not permitted, no entrained air] 4000 PSI Slab on steel deck, topping slabs [max w/c = 0.45, fly ash not permitted, no entrained air]
- 4500 PSI Piers, foundation walls, and exterior slabs [5%-7% air content] 4500 PSI Retaining walls, basement walls, pile caps, and grade beams

7000 PSI Non-shrink grout below baseplates All exterior concrete work shall have 5% to 7% air entrainment.

STEEL: (Fy)

- 60.000 PSI ASTM A615 grade 60 reinforcing
- 60,000 PSI ASTM A706 weldable reinforcing 50,000 PSI ASTM A992 wide-flange shapes 36,000 PSI ASTM A36 plates, channels, and angles, etc
- 50,000 PSI ASTM A500 grade C structural tubes (HSS) 46,000 PSI ASTM A500 grade C structural pipe (HSS)
- 92,000 PSI ASTM A325 high strength bolts 36,000 PSI ASTM F1554 threaded anchor rods 50,000 PSI ASTM A108 headed studs

FOUNDATION LOADS:

2,000 PSF soil bearing, based on soil report prepared by Geotek Engineering & Testing Services, Inc., dated April 17, 2019, (report # 19-225). See Geotek report for required site preparation. Helical Pulldown Micropiles, designed for unfactored load indicated on sheet SB102.

LATERAL EARTH PRESSURE:

35 PCF Active Lateral Earth Pressure (Equivalent Fluid Density)

STEEL HELICAL PILES:

- 1. Helical pile contractor shall submit calculations and shop drawings stamped by a licensed Professional Engineer in the state of South Dakota. Shop drawings shall be submitted for review and shall include helical pile components, corrosion protection system, pile attachment and helix details.
- 2. Inspection agency shall keep a record or log of each pile as installed showing location, top and bottom elevations, diameters, grout drop, date installed, type of strata encountered and any other pertinent information.

TEMPORARY BRACING:

Revisions:

VA FORM 08 - 6231

- 1. Provide temporary lateral support for all walls where grade varies on the two sides until slab has reached its design
- 2. Provide required temporary bracing for structural steel until permanent bracing and walls are in place.
- 3. Provide temporary bracing for all walls, concrete, masonry, light gage metal, or wood until they are of adequate design strength and are properly anchored in final form.
- 4. Provide temporary shoring for all existing walls, floors, and roof members until new construction is in place and properly anchored or cured in final form.
- 5. All temporary shoring is to be designed by a specialty shoring contractor, by a Professional Engineer licensed in the state of the project, at the expense of the contractor.
- 6. Shore all foundation walls as required before backfilling and compacting.
- 7. Contractor shall provide adequate bracing and shoring during all phases of construction and erection of the structure.

GENERAL FOUNDATION NOTES:

- 1. All foundation excavations, backfill, and compaction shall be inspected and certified by a qualified soils testing firm prior to the construction of any footings. All reports are to be submitted to Structural Engineer of Record in a timely manner.
- 2. Cross reference all architectural, mechanical, electrical, and structural drawings to assure proper dimensions and
- 3. All footing elevations are shown to top of footings, unless noted otherwise.
- 4. All footings are centered under walls or columns above, unless noted otherwise.
- 5. Continuous wall footings up through 1'-8" wide to be 10" thick. Footings over 1'-8" wide to be 12" thick, unless noted
- 6. Provide wall footing reinforcement as follows:

Masonry Notes for required lap length.

- Footings 2'-1" through 3'-0" wide = (3)-#5 cont. Footings 3'-1" through 3'-6" wide = (3)-#5 cont. & #5 @ 12" transv.
- See details for reinforcing in all footings over 3'-6" wide.

- - Cast against and permanently exposed to earth = 3"
 - Exposed to earth or weather: #5 bars and smaller = 1 1/2"
 - Not exposed to weather or in contact with ground:
 - Beams, girders and columns, primary reinforcement, ties, stirrups, or spirals = 1 1/2"
- 5. Provide corner bars at all corners and intersections of walls, grade beams, and edge beams. Corner bar to be the same size and spacing as all horizontal bars.
- 6. At openings in structural slabs or walls, provide a minimum of (2)-#6 bars each side of opening. Bars are to extend a minimum of 3'-0" beyond corners of openings, unless noted otherwise. Provide (1)-#5 x 4'-0" long diagonal bar at each corner of opening in each face of wall or slab.

- #4 @ 16" o.c. vert & #4 @ 16" o.c. horiz (each face)
- 12" concrete walls:
- 16" concrete walls:
- 8. Provide vertical control joints in exposed concrete walls at a maximum of 30'-0" intervals. See typical details for Control Joint and Construction Joint Detail.
- 9. No aluminum of any type shall be allowed in the concrete work, unless coated to prevent reaction with concrete.
- of construction.
- 11. Post-installed anchors in concrete shall be ICC approved for use in cracked concrete. Approved anchors shall be Hilti Kwik Bolt TZ Expansion Anchors (ESR-1917) or a Hilti HIT-HY 200 Adhesive Anchoring System (ESR-3187), unless noted otherwise. Install anchors in strict conformance with anchor manufacturer's instructions. Anchor substitutions shall not be made without written permission from the Structural Engineer of Record.
- 12. No pipe or conduit of any type shall be placed in structural concrete members without written approval from the Structural
- 13. Composite slabs and beams are designed to support the dead load of the wet concrete plus normal construction loads without requiring temporary shoring. Some deflection of the deck and beams will occur when the wet concrete is placed. The contractor shall include in the bid the cost of the additional concrete quantity caused by the deflection of the beams
- 15. Lap splice lengths in continuous reinforcing shall be tension lap splices and are shown below, unless noted otherwise on

D 0:	Top Bars	Other Bars		
Bar Size	Case 1	Case 2	Case 1	Case 2
#3	28"	42"	22"	32"
#4	37"	56"	29"	43"
#5	47"	70"	36"	54"
#6	56"	84"	43"	64"
#7	81"	122"	63"	94"
#8	93"	139"	72"	107"
#9	105"	157"	81"	121"
#10	118"	177"	91"	136"
#11	131"	196"	101"	151"
= 4000 PSI:				

f'c = 4000 PSI:				
	CLASS	B TENSION LAP SPLICE	LENGTH	
Top Bars Other Bars			Bars	
Bar Size	Case 1	Case 2	Case 1	Case 2
#3	24"	36"	19"	28"
#4	32"	48"	25"	37"
#5	40"	60"	31"	47"
#6	48"	72"	37"	56"
#7	70"	106"	54"	81"
#8	80"	121"	62"	93"
#9	91"	136"	70"	105"
#10	102"	153"	79"	118"
#11	113"	170"	87"	131"

- placement of all anchor rods, inserts, etc.

- Footings up through 2'-0" wide = (2)-#5 cont.

- 7. Provide 90 degree bend in all footing dowels. Cast dowels in footings for columns, piers, and walls above. Dowels to be the same number and size as the vertical reinforcing, unless noted otherwise. See General Concrete Notes or General
- 8. Rebar and anchor rods to be securely tied in place prior to placing concrete (i.e. no "wet-sticking" is allowed).

GENERAL CONCRETE NOTES:

- 1. Concrete construction shall comply with the provisions of the "Building Code Requirements for Structural Concrete,"
- 2. The "ACI Detailing Manual" shall govern detailing and fabrication of all reinforcing steel, unless noted otherwise.
- 3. Reinforcing steel supplier to provide all accessories, chairs, spacing bars, and supports necessary to secure steel in accordance with "Manual of Standard Practice" by the Concrete Reinforcing Steel Institute. Clay brick is not allowed.
- 4. Provide minimum clear concrete cover for all reinforcement as follows:

 - #6 bars and larger = 2"
 - Slabs, walls, & joists (#3 to #11 bars) = 3/4"

- 7. Provide minimum concrete wall reinforcing as follows: (unless noted otherwise)
- 6" & 8" concrete walls: #4 @ 16" o.c. vert & #4 @ 10" o.c. horiz (center in wall)
- 10" concrete walls:
- #4 @ 16" o.c. vert & #4 @ 12" o.c. horiz (each face)
- #4 @ 16" o.c. vert & #4 @ 12" o.c. horiz (each face)
- 10. Maximum outside diameter of embedded conduit shall be no larger than 1/3 of the slab thickness. This restriction applies to the total height at conduit crossings. The conduit shall be placed such that it does not significantly impair the strength

- 14. Do not weld rebar, unless Weldable Rebar is provided and its use is approved by the Structural Engineer of Record.
- drawings or details:

f'c = 3000 PSI:

CLASS B TENSION LAD SDLICE LENGTH

	CLASS	B TENSION LAP SPLICE	E LENGTH	
	Top Bars		Other Bars	
Bar Size	Case 1	Case 2	Case 1	Case 2
#3	22"	33"	17"	25"
#4	29"	43"	22"	33"
#5	36"	53"	28"	41"
#6	43"	65"	33"	50"
#7	62"	95"	48"	73"
#8	72"	108"	55"	83"
#9	81"	122"	62"	94"
#10	91"	137"	70"	105"
#11	101"	152"	78"	117"

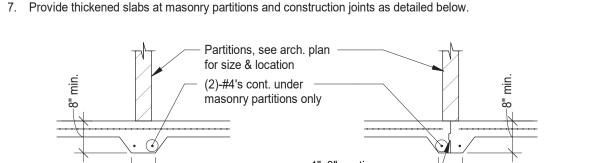
- 1. Tables are for normal weight concrete with Grade 60 uncoated reinforcing bars. For lightweight aggregate, multiply the values in the table by 1.33.
- 2. Top bars are horizontal bars with more than 12" of concrete cast below the bars.
- 3. Compression lap splices (only where indicated on drawings) for Grade 60 uncoated reinforcing bars shall be 30 times the
- 4. Cases 1 and 2 are defined as follows:

f'c = 5000 PSI:

- Beams and columns: Case 1: Concrete cover at least 1.0 times the bar diameter and center-to-center spacing of at least 2.0 times the Case 2: Concrete cover less than 1.0 times or center-to-center spacing less than 2.0 times the bar diameter.
- Case 1: Concrete cover at least 1.0 times the bar diameter and center-to-center spacing at least 3.0 times the bar diameter. Case 2: Concrete cover less than 1.0 times the bar diameter or center-to-center spacing less than 3.0 times the

bar diameter.

- **CONCRETE SLAB AND JOINT NOTES AND DETAILS:** 1. Control Joints (C.J.) - Locate saw cut control joints at column centerlines and at the following maximum spacing to create approximately square panels
- a. Concrete slabs on grade: i. 4"-5" thick slab = 12'-0"
- Coordinate control joint layout with floor finish requirements. b. Control joint depth to be 1", using an early entry saw. c. Cut control joints with an early entry saw as soon as possible without damage to the slab surface.
- 2. Provide 6x6-W1.4xW1.4 W.W.F. in all slabs on grade, unless noted otherwise. All mesh to be lapped a minimum of 12". Provide prefabricated sheets in lieu of rolled mesh. Reinforce with (2)-#5 x 3'-0" long at all re-entrant (inside) corners.
- 3. Place slab reinforcing between 1/4 and 1/3 of slab thickness down from top of slab.
- 4. Coordinate all floor finishes, slopes, recesses, floor drains, gutters, etc. with all disciplines (arch., mech., etc.). 5. Provide a preformed isolation joint in concrete slab at columns. The isolation joint can be either a circular or diamond
- 6. Do not provide control joints in structural slabs, slabs on metal deck, or precast topping, unless noted otherwise.



minimum of 1'-0"

(STOP MESH AT CONSTRUCTION JOINT)

PARTITION FOOTING

minimum of 1'-0"

- **TYPICAL LINTEL TYPES AND NOTES:** 1. Verify size and location of all mechanical, U.V., U.H., louver, and duct openings with mechanical contractor.
- 2. For all openings through masonry walls not shown, including mechanical and electrical openings, provide one of the following: (unless noted otherwise)
- (1) L 3 1/2" x 3 1/2" x 1/4" for each 4" thickness of wall for spans up to 4'-0".
- (1) L 6" x 3 1/2" x 5/16" (LLV) for each 4" thickness of wall for spans up to 6'-0".

(1) L 5" x 3 1/2" x 5/16" (LLV) for each 4" thickness of wall for spans up to 5'-0".

3. All steel lintel beams to bear a minimum of 8" on grouted or solid masonry, unless noted otherwise. All steel lintel angles to bear a minimum of 6" on solid or grouted masonry, unless noted otherwise.

4. All lintels in exterior walls to be hot-dipped galvanized, unless noted otherwise.

GENERAL STEEL NOTES:

a. Steel angle lintels:

- 1. Construction of structural steel shall comply with the provisions of "AISC 360-16 Specification for Structural Steel Buildings" and "AISC 341-16 Seismic Provisions for Structural Steel Buildings."
- 2. All shop connections shall be welded or bolted, field connections shall be bolted, unless noted otherwise. Bolted connections shall be Bearing Type (snug-tightened) and shall be made with a minimum of 3/4"ø ASTM A325-N Bolts. Direct-Tension Indicators are acceptable substitutions.
- 4. Before encasing steel columns in concrete or masonry, paint column bases and tops of anchor rods with asphaltic paint. 5. The structural fabricator shall furnish all plates and angles cast in bond beams, concrete walls, or columns to support

3. All welds as per latest specifications of the AWS - E70xx electrodes.

- steel joists, beams, and steel deck. 6. 'C' denotes beam is continuous over columns, 'S' denotes beam simple shear splice.
- 7. All steel beams shall be true to line and elevation, column base plates grouted, and anchor rods tight before any loads

13. The top of all beams receiving shear studs shall not be painted.

required number of bolts, weld requirements, etc., unless specifically noted.

- 8. All column base and cap plates to be welded around all sides. 9. All welds not specified are 3/16" fillet weld, continuous and/or all around.
- 10. Structural fabricators shall show all welding requirements on structural steel shop drawings.
- 11. Fabricator shall select AISC simple shear connections for composite beams capable of carrying the reaction load indicated or the reaction load calculated and based on tributary area or at a minimum 75% of the total shear capacity. Connection design shall also be check for blast reaction given on sheet SG000.
- 12. Cuts, holes, or openings required in structural steel members for the work of other trades shall be shown on the shop drawings. Burning of holes and cuts in structural steel members in the field shall not be allowed, except by written permission from the Structural Engineer of Record.
- 14. The contractor shall provide 4000 pounds of structural steel contingency material to be fabricated and erected as directed by the Structural Engineer of Record. Cost of material, labor, delivery, and associated services are to be included in the 15. All connections not specifically detailed shall be designed by a Professional Engineer licensed in the state where the

project is located. Detailing shall be performed using rational engineering design and standard practice in conformance

with the contract documents. The general details shown on the drawings are approximate only and do not indicate the

16. Shear stud connectors shall be manufactured by Nelson Stud Welding Co. or equal conforming to ASTM A108, and shall be field applied with automatic welding equipment through the composite steel deck with the use of a proper ferrule.

17. Location, type, diameter, length, and spacing of shear stud connectors shall be detailed on the shop drawings.

STEEL DECK NOTES:

- I. All steel decking shall comply with the specifications of the Steel Deck Institute (SDI). Thickness, type, and properties of decks shall be as shown on the drawings.
- 2. All steel deck shall span a minimum of three spans, unless otherwise approved.
- 3. Field weld 1 1/2" steel roof deck to supporting members with 5/8"ø puddle welds at 36/4 pattern. Where areas of warped deck occur, field weld steel deck maximum 6" o.c. at all supports. Typical, unless noted otherwise.
- 4. 1 1/2" steel roof deck shall have; (1)-#10 TEK screw side lap connector installed between adjacent supports (unless noted otherwise).
- 5. Composite steel deck with concrete slabs shall be welded to all supporting members with 5/8"ø puddle welds at 36/4 pattern. For deck units with spans greater than 5'-0", sidelaps and perimeter edges of units between span supports shall
- a) #10 self-drilling screws b) Crimp or button punch c) Arc puddle welds 5/8" minimum visible diameter, or minimum 1" long fillet weld.

be fastened at intervals not exceeding 36" o.c., using one of the following methods:

- 6. See plans and details for composite deck thickness, depth, and profile. All composite steel deck to be galvanized with
- Steel conform deck shall be attached at all supports sufficiently to prevent movement. Steel deck fasteners are not
- required for conform decks supporting concrete stoop slabs. 8. For steel conform deck supporting interior floor slabs attach deck to supporting members with 5/8" puddle welds per the
- following pattern layout: 0.6C Conform Deck – 30/4 weld pattern 1.0C Conform Deck – 33/4 weld pattern
 - 1.3C Conform Deck 32/4 weld pattern 2.0C Conform Deck – 36/4 weld pattern

ROOF BEAM/ GIRDER BLAST REACTION TABLE

DESCRIPTION	SPAN	END REACTION (kips, LRFD)
W12x14 BEAM	8'-0" TO 12'-2"	50
W12x26 BEAM	12'-2"	90
W14x22 BEAM	19'-8"	55
W16x36 BEAM	12'-2"	140
W18x35 BEAM	29'-5"	78
W18x46 BEAM	20'-0"	115
W12x19 GIRDER	17'-0"	31
W14x22 GIRDER	8'-1" TO 18'-3"	44
W14x30 GIRDER	19'-2"	42
W16x26 GIRDER	15'-10" TO 20'-0"	50
WYOAZO GINDLIN	21'-1" TO 23'-10"	76
W18x35GIRDER	20'-0"	54
W21x44 GIRDER	20'-0"	145
W24x55 GIRDER	26'-7"	145
		I .

ROOF MEMBER REACTION TABLE NOTES:

- CONNECTIONS MUST BE DESIGNED FOR THE REACTION LOADS LISTED IN THE TABLES AT A MINIMUM. THESE SHOULD NOT BE COMBINED WITH OTHER LOADS (I.E., BLAST DESIGN COMBO=1.0 * BLAST)
- REACTIONS ARE VERTICAL AND ARE TO BE CONSIDERED IN THE UP AND DOWN

REACTIONS ARE EQUIVALENT STATIC REACTIONS REPORTED AS ULTIMATE LOADS

FOR CONNECTION DESIGN PER LRFD. IF ASD IS USED, DIVIDE THE REACTIONS B

YA FACTOR OF 1.5. USE PROPER MATERIAL-AND LIMIT-STATE-SPECIFIC REDUCTION FACTOR (ϕ OR Ω) PER CODE FOR CONNECTION DESIGN.

PX **--**— PIER MARK

CX COLUMN MARK

GROUT OR DRYPACK OR SAND

STRUCTURAL SYMBOL LIST				
GENERAL SYMBOLS:				
SYMBOL DESCRIPTION DETAIL REFERENCE				
(100'-0")	TOP OF ELEVATION	N/A		

DESCRIPTION

o	STEP IN FOOTING	1/SB200		
STEEL SYMBOLS:				
SYMBOL	DESCRIPTION	DETAIL REFERENCE		
	STEEL DECK (DIRECTION)	SEE PLAN NOTES		
[##]	HEADED STUD ANCHORS (HSA) ON BEAM			
⊣ ⊢	BEAM SPLICE	3/SF220		
—	MOMENT CONNECTION			
/ \	BRACE ABOVE	SEE FRAME ELEVATION		
/ \	DD A OF DELOW			

DEGREE DIAMETER EXISTING ANCHOR BOLT ARCHITECT, -URE, -URAL ARCH B.O. BOTTOM OF BEAM FLANGE WIDTH BRACE FRAME BOUNDARY NAILING BOTT BOTTOM BETWEEN BTWN COLD FORM STEEL FRAMING CFSF CENTER OF GRAVITY OF THE TENDON **COMPLETE JOINT PENETRATION WELD** CLR CENTERLINE CONCRETE MASONRY UNIT CMU COL CONC COLUMN CONCRETE CONN CONNECTION CONST CONSTRUCTION CONTINUOUS CONT COORD COORDINATION DIAMETER DEAD LOAD DETAIL DET DRAWING DWG DOWEL DWL FACH EACH FACE EFFECTIVE EFF FLEVATION ELECTRICAL ELEC **EMBEDMENT EMBED** EDGE NAILING EDGE OF DECK EOD EOS EDGE OF SLAB FOUAL EQUIPMENT EQUIP ETCETERA ETC EACH WAY EXPANSION EXP FXTFRIOR EXT CONCRETE COMPRESSIVE STRENGTH FDN FOUNDATION FIELD NAILING FOOTING FTG YIELD STRESS GAGE OR GAUGE GALVANIZED HORIZONTAL HORIZ HEADED STUD ANCHOR HIGH STRENGTH BOLT KILOPOUND (1,000 POUNDS) KIPS PER SQUARE FOOT KIPS PER SQUARE INCH KSI I FNGTH POUNDS LIVE LOAD LONG LEG HORIZONTAL LONG LEG VERTICAL LONGITUDINAL LONG. LONG SIDE HORIZONTAL LONG SIDE VERTICAL □LIGHTWEIGH MAXIMUM MAX MECH MECHANICAL MANUF MANUFACTURER MINIMUM MIN NOT IN CONTRACT NOT TO SCALE ON CENTER OPPOSITE HAND OPENING OPNG ORIENTED STRAND BOARD POUNDS PER CUBIC FOOT PENTHOUSE P.H. PARTIAL JOINT PENETRATION WELD PJP

STRUCTURAL ABBREVIATION KEY

NUMBER OR POUNDS

ABBR: DESCRIPTION:

SF#(X'-X") ─ FOOTING MARK (TOP ELEVATION) **FOUNDATION SYMBOLS:**

VIEW KEY

NAME LEVEL NAME
100'-0" ELEVATION

LINE TYPE KEY:

----- NEW WORK

1 INDICATES NOTE USED TO DESCRIBE

SHEET AND/OR DETAIL

- INDICATES DIRECTION OF TRUE NORTH

INDICATES SIMILAR DETAIL REFERENCED

- DETAIL REFERRED TO BY SECTION CUT

- PLAN OR DETAIL NUMBER

- PLAN OR DETAIL NAME

- PLAN OR DETAIL SCALE

IN MULTIPLE LOCATIONS

S300 /- SHEET DETAIL IS LOCATED ON

(DARK SOLID LINE/LINE WEIGHT WILL VARY)

(HALFTONED SOLID LINE/LINE WEIGHT WILL VARY)

---- NEW WORK BELOW OR BEYOND VIEW

- EXISTING WORK TO REMAIN

(HALFTONED LIGHT SOLID LINE)

(DARK DASH LINE)

(DARK DASH LINE)

NON STRUCTURAL

---- EXISTING TO BE REMOVED

— - — - — GRID OR CENTERLINE

MATERIAL LEGEND:

SYMBOL

ADDITIONAL INFORMATION ABOUT

PRECAST CONCRETE

DETAIL REFERENCE

WORK REQUIRED, SPECIFIC TO THE

STEEL SYM	BOLS:	
SYMBOL	DESCRIPTION	DETAIL REFERENCE
	STEEL DECK (DIRECTION)	SEE PLAN NOTES
[##]	HEADED STUD ANCHORS (HSA) ON BEAM	
⊣ ⊢	BEAM SPLICE	3/SF220
—	MOMENT CONNECTION	
/	BRACE ABOVE	SEE FRAME ELEVATION
/ \	BRACE BELOW	SEE FRAME ELEVATION

STRUCTURAL SHEET INDEX GENERAL NOTES **TESTING SCHEDULES** PIPE BASEMENT FOUNDATION PLAN GROUND LEVEL FOUNDATION PLAN GROUND LEVEL COLUMN LOADS ENLARGED PLANS SECTIONS - FOUNDATION SECTIONS - FOUNDATION FIRST FLOOR & ROOF FRAMING PLAN ROOF FRAMING PLAN SECTIONS - FLOOR SECTIONS - ROOF SECTIONS - ROOF BRACE FRAME ELEVATIONS & SECTIONS GRAND TOTAL: 14

Project Number Project Title 438-460 **GENERAL NOTES BID DOCUMENTS** CONSTRUCT NEW SPS **Building Number Drawing Number** Sioux Falls, SD. **FULLY SPRINKLERED**

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| U.S. Department

of Veterans

Drawing Title

Checked **Issue Date** MPM/TGL MAQ

SG000

METAL / COLD-FORM STUD

POUNDS PER SQUARE INCH POST-TENSION, -ED, -ING RADIUS REINFORCING, -MENT, -ED REINF REQUIRED ROOF TOP UNIT RTU SLIP CRITICAL SCHED SCHEDULE SEISMIC FORCE-RESISTING SYSTEM SFRS SIMILAR SIM SNOW LOAD SHEET METAL SCREW S.M.S. SPACE(S)

SPECS | SPECIFICATION(S)

THICK

TYPICAL

TRANS TRANSVERSE

THK

TYP

VERT VERTICAL VERIFY IN FIELD VIF W/ WORK POINT WEIGHT WWR WELDED WIRE REINFORCING

UNLESS OTHERWISE NOTED

SQUARE STIFFENER STIFF STEEL STL SYMMETRICAL SYM TOP AND BOTTOM T&B T.O. TOP OF PRE-TENSIONED BOLT TEMPERATURE BEAM FLANGE THICKNESS

POUNDS PER LINEAR FOOT POUNDS PER SQUARE FOOT

STATEMENT OF SPECIAL INSPECTION:

Special Inspections and Testing requirements per Chapter 17 of the IBC in addition to Section 110 of the IBC (Inspection preformed by the Building Official). See Specs. for additional information.

Structural Testing & Special Inspection Program Summary Schedule				
IBC Section	Material	Type of Inspector	Report Frequency	
1704.2.5	Shop Fabrication	SI-S	Upon Completion	
1705.2	Steel	SI-S	Weekly	
1705.3	Concrete	SI-S	Weekly	
1705.6	Soils/Earthwork	SI-T	Upon Completion	
1705.7-1705.9	Deep Foundation	SI-T	Upon Completion	

SI-S Special Inspector-Structural SI-T=Special Inspector-Technical

1703.1 - APPROVALS

Agency must be approved by the Building Official or AHJ.

2. Agency must be independent of the contractor responsible for work and disclose possible conflicts of interests.

1704.2.4 - SPECIAL INSPECTOR RESPONSIBILITIES:

- Submit inspection reports to the Building Official, Architect, Engineer of Record (EOR), and Contractor, stating the work was or was not in conformance with construction documents.
- 2. Discrepancies shall be brought to the immediate attention of the contractor for correction.
- 3. If discrepancy is not corrected, it shall be brought to the attention of the building official and EOR in a timely manner to provide remediation or acceptance prior to the completion of work.
- 4. Submit a final report documenting required special inspections and correction of any discrepancies noted.

1704.2.5 - FABRICATION:

Where fabrication of structural members and assemblies are being fabricated on the premises of a fabricator's shop, special inspection is required of the fabricated item.

Note: Where Special Inspection and Testing of Shop Fabricated Components is required, it shall conform to the Special Inspection and Testing required in the field for the material specific section the component is fabricated from.

Exception: Special Inspection of the Fabricator's shop is not required if approved per Section 1704.2.5.2.

	TABLE 1705.6 REQUIRED VERIFICATION AND INSPECTION SOILS				
	Verification and Inspection Task	Continuous During Task Listed	Periodically During Task Listed		
1.	Verify materials below footings are adequate to achieve the design bearing capacity.		Х		
2.	Verify excavations are extended to proper depth and have reached proper material.		X		
3.	Perform classification and testing of controlled fill materials.		X		
4.	Verify use of proper materials, densities and lift thicknesses during placement and compaction of controlled fill.	X			
5.	Prior to placement of compacted fill, observe subgrade and verify that site has been prepared properly.		X		

	TABLE 1705.7 REQUIRED VERIFICATION AND INSPECTION OF DEEP FOUNDATION ELEMENTS				
Verification and Inspection Task		Continuous During Task Listed	Periodically During Task Listed		
1.	Verify element materials, sizes and lengths comply with the requirements.	X			
2.	Determine capacities of test elements and conduct additional load tests, as required.	X			
3.	Observe driving operations and maintain complete and accurate records for each element.	Х			
4.	Verify placement locations and plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and document any damage to foundation element.	X			
5.	For steel elements, perform additional inspections in accordance with Section 1705.2.				
6.	For concrete elements and concrete-filled elements, perform additional inspections in accordance with Section 1705.3.				
7.	For specialty elements, perform additional inspections as determined by the registered design professional in responsible charge.				

1705.9 - HELICAL PIERS

Special Inspections shall be performed continuously during installation of helical piles and shall include the following:

--- Equipment Used, Pile Dimensions, Tip Elevations, Final Depth, Final Installation Torque, other info.

Verifi	cation and Inspection	Continuous Periodic		Referenced Standard (a)	IBC Reference	
1.	Inspection of reinforcing steel, including prestressing tendons, and placement.		Х	ACI 318: 3.5, 7.1 - 7.7	1910.4	
2.	Inspection of reinforcing steel welding in accordance with Table 1705.2.2, item 2b.			AWS D1.4, ACI 318: 3.5.2		
3.	Inspection of anchors cast in concrete where allowable loads have been increased or where strength design is used.		Х	ACI 318: 8.1.3, 21.2.8	1908.5, 1909.1	
4.	Inspection of anchors post-installed in hardened concrete members (b).		Х	ACI 318: 3.8.6, 8.1.3, 21.2.8	1909.1	
5.	Verifying use of required design mix.		Х	ACI 318: Ch. 4, 5.2 - 5.4	1904.2, 1910.2, 1910.3	
6.	At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	Х		ASTM C 172, ASTM C 31, ACI 318: 5.6, 5.8	1910.10	
7.	Inspection of concrete and shotcrete placement for proper application techniques.	Х		ACI 318: 5.9, 5.10	1910.6, 1910.7, 1910.8	
8.	Inspection for maintenance of specified curing temperature and techniques.		Х	ACI 318: 5.11 - 5.13	1910.9	
9.	Inspection of prestressed concrete:					
	a. Application of prestressing forces.	Х		ACI 318: 18.20 ACI 318: 18.18.4		
	b. Grouting of bonded prestressing tendons in the seismic force-resisting system.	Х				
10.	Erection of precast concrete members.		Х	ACI 318: Ch. 16		
11.	Verification of in-situ concrete strength, prior to stressing of tendons in post- tensioned concrete and prior to removal of shores and forms from beams and structural slabs.		Х	ACI 318: Ch. 6.2		
12.	Inspect formwork for shape, location and dimensions of the concrete member being formed.		Х	ACI 318: Ch. 6.1.1		

For SI: 1 inch=25.4mm

a. Where applicable, see also Section 17.05.11, Special inspections for seismic resistance.

b. Specific requirements for special inspection shall be included in the research report for the anchor issued by an approved source in accordance with ACI 355.2 or other qualification procedures. Where specific requirements are not provided special inspection requirements shall be specified by the registered design professional and shall be approved by the building official prior to the commencement of the work.

1. Non-structural concrete slabs supported directly on the ground.

2. Concrete patios, driveways and sidewalks on grade.

In addition to the requirements below also comply w/ AISC 360-10 Chapter N

Welding Inspection Tasks	Bolting Inspection Tasks			
Welding procedure specifications (WPSs) available	Р	Manufacturer's certifications available for fastener materials		
Manufacturer certifications for welding consumables available	Р	Fasteners marked in accordance with ASTM requirements		
Material identification (type/grade)	0	Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)		
Welder identifications system (1)	0	Proper bolting procedure selected for joint detail		
Fit-up of groove welds (including joint geometry)	0	Connecting elements, including the appropriate faying surface condition and hole		
Configuration and finish of access holes	0	preparation, if specified, meet applicable requirements		
Fit-up of fillet welds	0	Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used		
Check welding equipment		Proper storage provided for bolts, nuts, washers and other fastener components		
Use of qualified welders	0	Fastener assemblies, of suitable condition, placed in all holes and washers (if		
Control and handling of welding consumables	0	required) are positioned as required.		
No welding over cracked tack welds	0	Joint brought to the snug-tight condition prior to the pretensioning operation		
Environmental conditions	0	Fastener component not turned by the wrench prevented from rotating		
WPS followed	0	Fasteners are pretensioned in accordance with the RCSC Specification, progressing systematically from the most rigid point toward the free edges.		
Welding techniques	0	Document acceptance or rejection of bolted connections		
Welds cleaned	0	0 - Observe these items on a random basis. Operations need not be		
Size, length and location of welds	Р	delayed pending these inspections.		
Welds meet visual acceptance criteria	Р	P - Perform these tasks for each welded joint or steel member		
Arc strikes	Р	N5.7		
k-area (2)	Р	Anchor Bolts and other embedded items supporting structural steel, verify diameter, g		
Backing removed and weld tabs removed (if required)	Р	type, length of embedded item, and the embedment depth prior to placing concrete.		
Repair activities	Р			
Document acceptance or rejection of welded joint or member P		TABLE N6.1		
(1) The fabricator or erector, as applicable, shall maintain a system by whall welded a joint or member can be identified. Stamps, if used, shall be the		Inspection of Steel Elements of Composite Construction Prior to Concre Placement		
(2) When welding of double plates, continuity plates or stiffeners has bee		Placement and installation of steel deck		
area, visually inspect the web k-area for cracks within 3 in. (75mm) of the weld.		Placement and installation of steel headed stud anchors		

P - Perform these tasks for each welded joint or member

0 - Observe these items on a random basis. Operations need not be delayed pending these

Non-destructive Testing (NDT) of Welds shall be performed in accordance with AWS D1.1/D1.1M based on the following criteria:

N5.5

For structures in Risk Category III or IV, Ultrasonic Testing (UT) shall be performed on all Complete Joint Penetration (CJP) groove welds for materials 5/16" thick or greater. Structures in Risk Category II, UT shall be performed on 10% of CJP groove welds for materials 5/16" thick or greater.

When flange or web thickness exceeds 2", thermally cut access holes shall be tested using Magnetic Particle Testing (MT) or Penetrant Testing (PT), any crack is unacceptable.

Welded joints requiring soundness per Appendix 3, Table A-3.1 shall be tested by Radiographic Testing (RT) or UT. Reduction in the rate of UT is prohibited.

Reduction rate for UT - Where the initial rate for UT is 100%, the NDT rate for an individual welder is permitted to be reduced to 25% provided the reject rate is 5% or less based on a minimum of 40 welds tested. For continuous welds over 3', each 12" increment shall be considered on weld. Increase rate for UT - Where the initial rate for UT is 10%, the NDT rate for an

individual welder shall be increased to 100% if the reject rate is over 5% based on a minimum of 20 welds tested. Rate may be reduced if reduction rate criterion is met.

Verif	Verification and Inspection			Continuous	Periodic	Referenced Standard (a
Material verification of cold-formed steel deck						
	a.		entification markings to conform to ASTM standards ecified in the approved construction documents		Х	Applicable ASTM materia standards
	b.	Ma	anufacturer's certified test reports		Х	
2.	Insp	ection	of welding:			
	a.	Со	ld-formed steel deck:			
		1.	Floor and roof deck welds		Х	AWS D1.3
	b.	Re	inforcing steel:			
		1.	Verification of weldability of reinforcing steel other than ASTM A706		Х	
		2.	Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special structural walls of concrete and shear reinforcement.	Х	Х	AWS D1.4 ACI 318: Section 3.5.2
		3.	Shear reinforcement	Х		
		4.	Other reinforcing steel		X	

OPEN-WEB STEEL JOISTS						
rification and Inspection	Continuous	Periodic	IBC Reference			
ify size and grade of joists upon placement		Х				
er setting, verify proper bearing length is provided		Х				

0 - Observe these items on a random basis. Operations need not be delayed pending these inspections.					
P - Perform these tasks for each welded joint or steel member					
N5.7					
Anchor Bolts and other embedded items supporting structural steel, verify diameter, grade, type, length of embedded item, and the embedment depth prior to placing concrete.					
TABLE N6.1					
Inspection of Steel Elements of Composite Construction Prior to Concrete Placement					
Placement and installation of steel deck	Р				
Placement and installation of steel headed stud anchors	Р				

0 - Observe these items on a random basis. Operations need not be delayed pending these inspections.

P - Perform these tasks for each steel member.

Document acceptance or rejection of steel elements

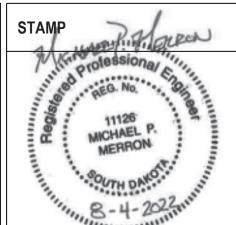
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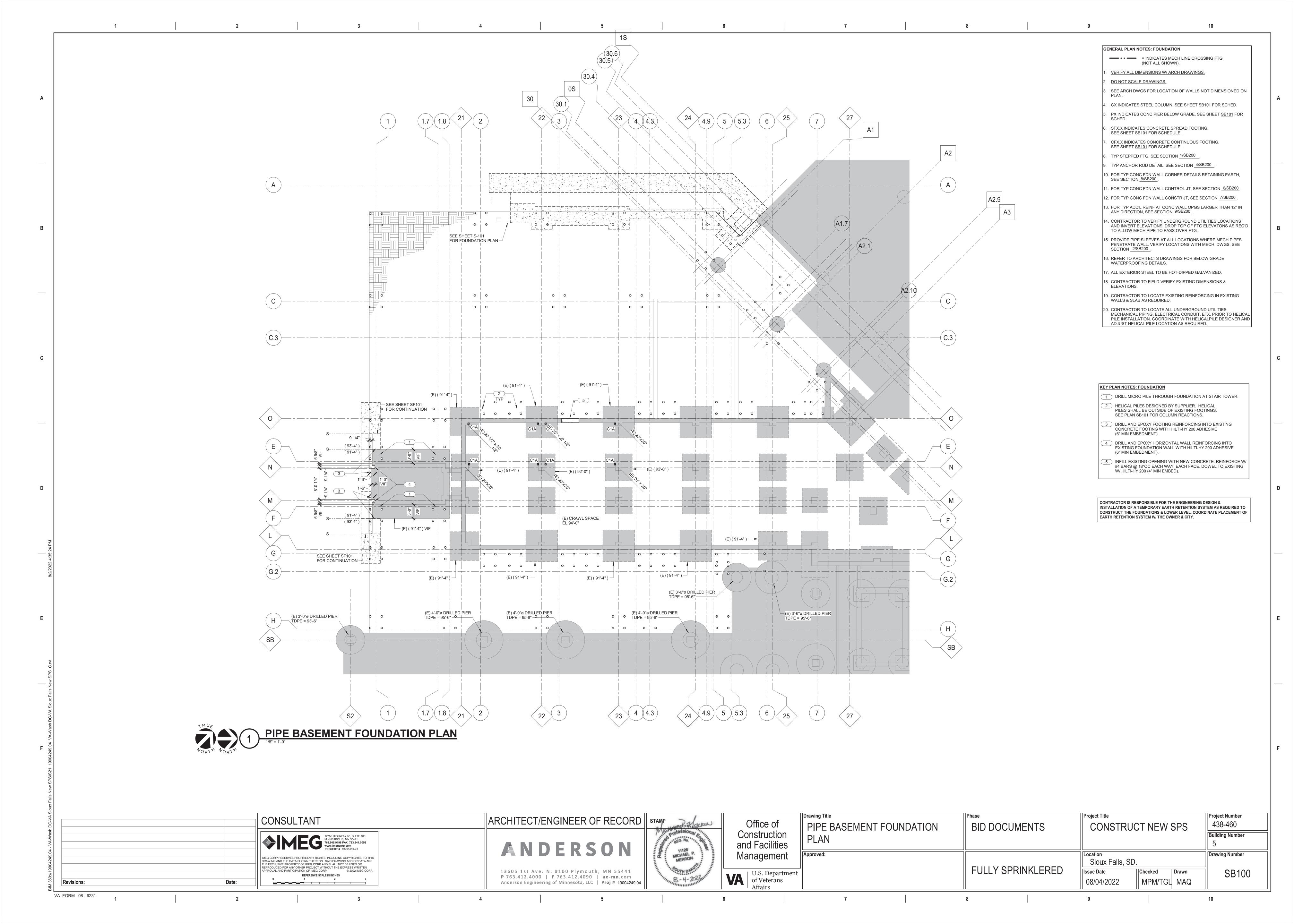


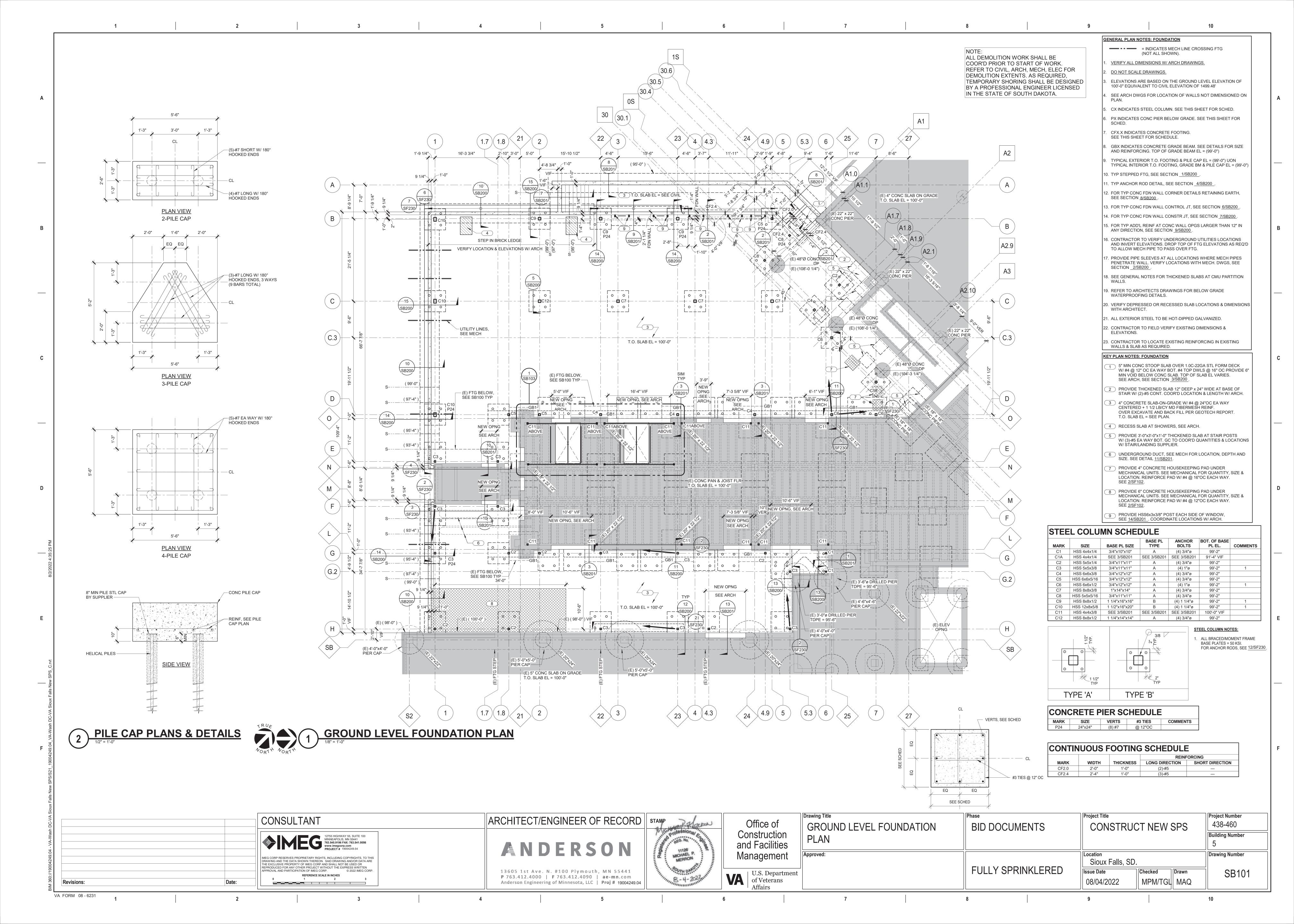
Office of

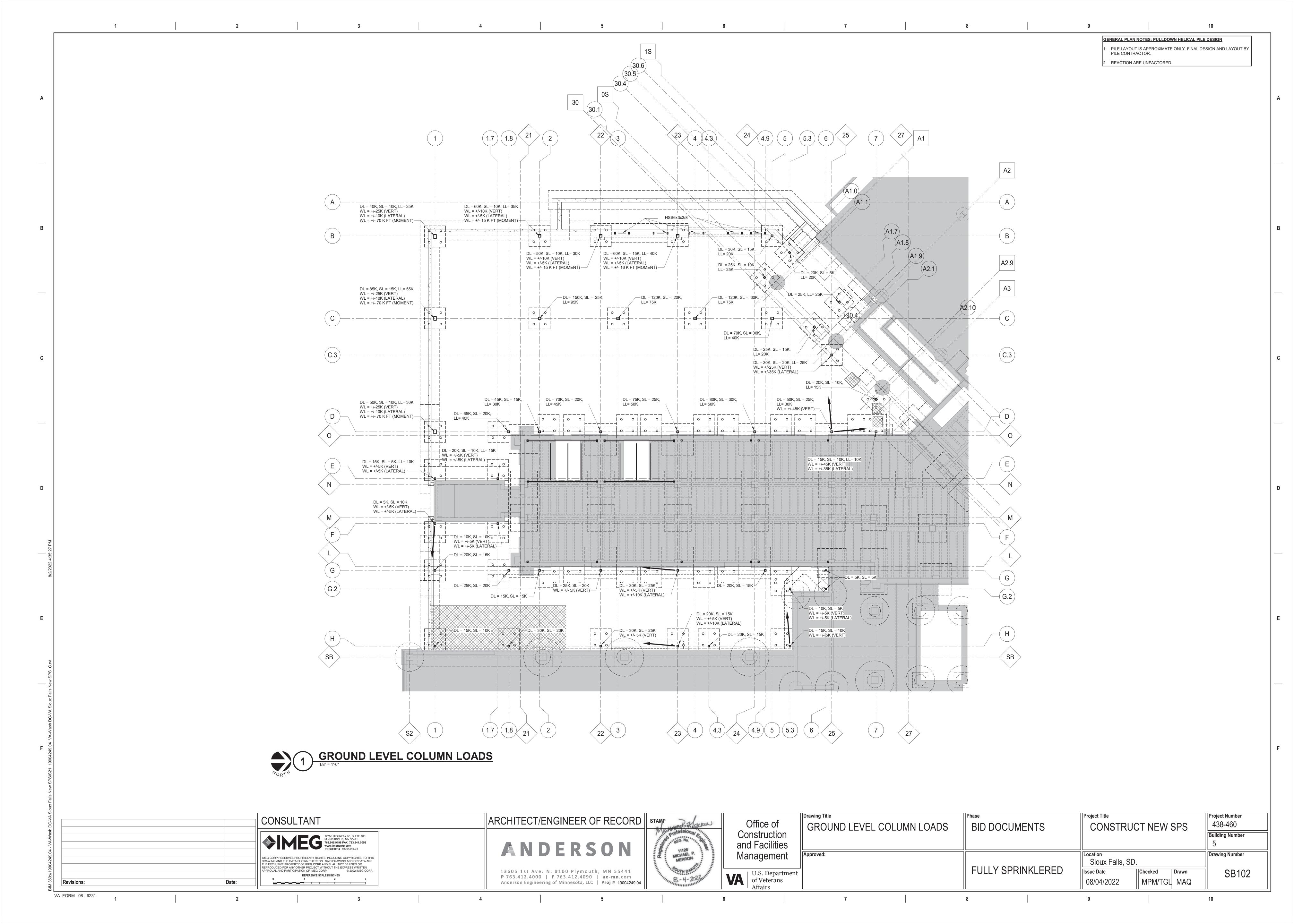
Construction

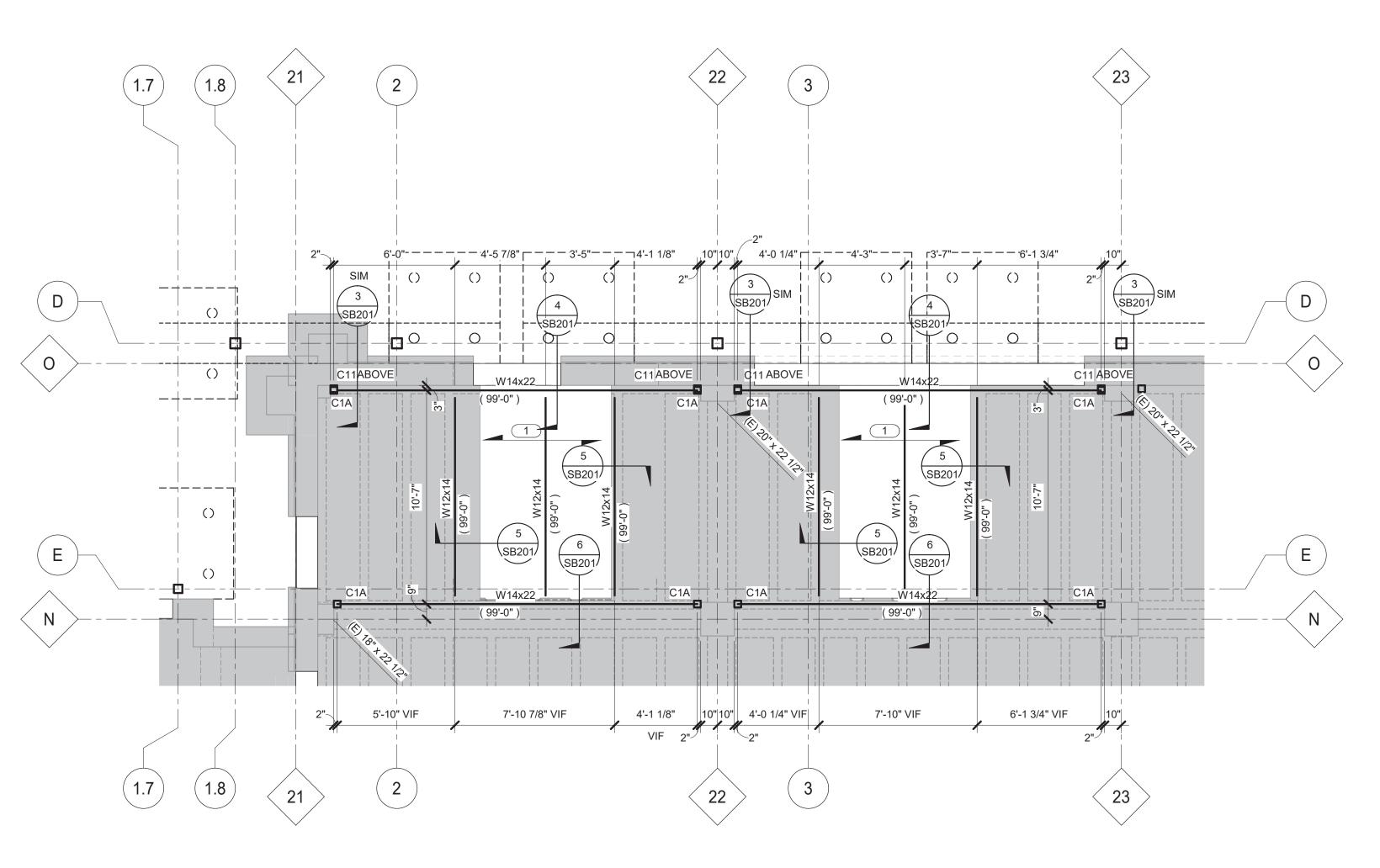
and Facilities Management VA U.S. Department of Veterans Affairs

Drawing Title Project Title Project Number 438-460 TESTING SCHEDULES BID DOCUMENTS CONSTRUCT NEW SPS Building Number Drawing Number Sioux Falls, SD. FULLY SPRINKLERED Issue Date | Checked | Drawn SG001 08/04/2022 MPM/TGL MAQ









LEVEL 01 CART WASH FLOOR FRAMING PLAN

1/4" = 1'-0"

GENERAL PLAN NOTES: FLOOR

VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

DO NOT SCALE DRAWINGS.

FOR SCHEDULES SEE SHEET <u>SB101</u>.

FOR PERMISSABLE CONSTRUCTION JOINT LOCATIONS IN COMPOSITE SLABS, SEE <u>2/SF210</u>.

FOR OPENINGS IN COMPOSITE SLABS, SEE 1/SF210 . FOR OPENINGS LARGER THAN 24", PROVIDE W10X12 FRAME AROUND OPG. VERIFY SIZE, LOCATION AND QTY WITH ARCH & MECH.

VERIFY ALL OPENINGS IN SLAB WITH ARCHITECT & MECHANICAL.

VERIFY EXISTING DIMENSIONS & ELEVATIONS.

CONTRACTOR TO LOCATE REINFORCING IN EXISTING WALLS & SLABS AS REQUIRED.

KEY PLAN NOTES: FLOOR FRAMING FRAMING

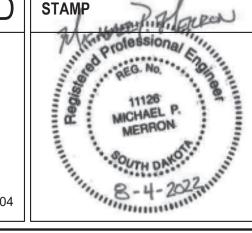
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4" TOTAL COMPOSITE CONCRETE FLOOR SLAB W/ #4 @ 12" OC EA WAY DOWELED TO EXISTING SLAB, OVER 1.0C-20 GA FORM DECK OR APPROVED EQUAL (3" CONC SLAB + 1" DECK = 4" MIN TOTAL SLAB THICKNESS).

T.O. SLAB EL =99'-4" (SLOPE SLAB PER ARCH DRAWINGS) 4" MIN SLAB THICKNESS.

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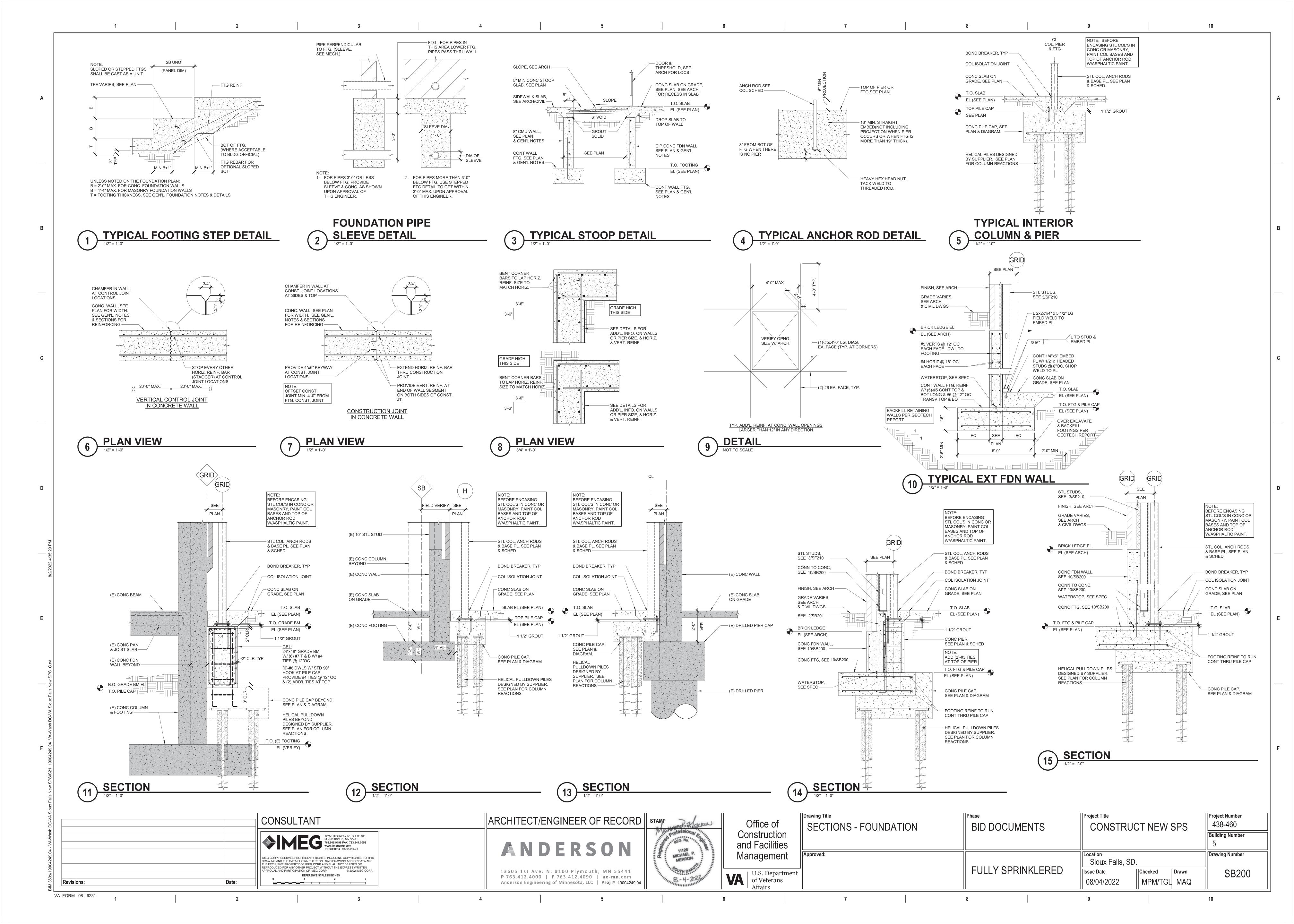
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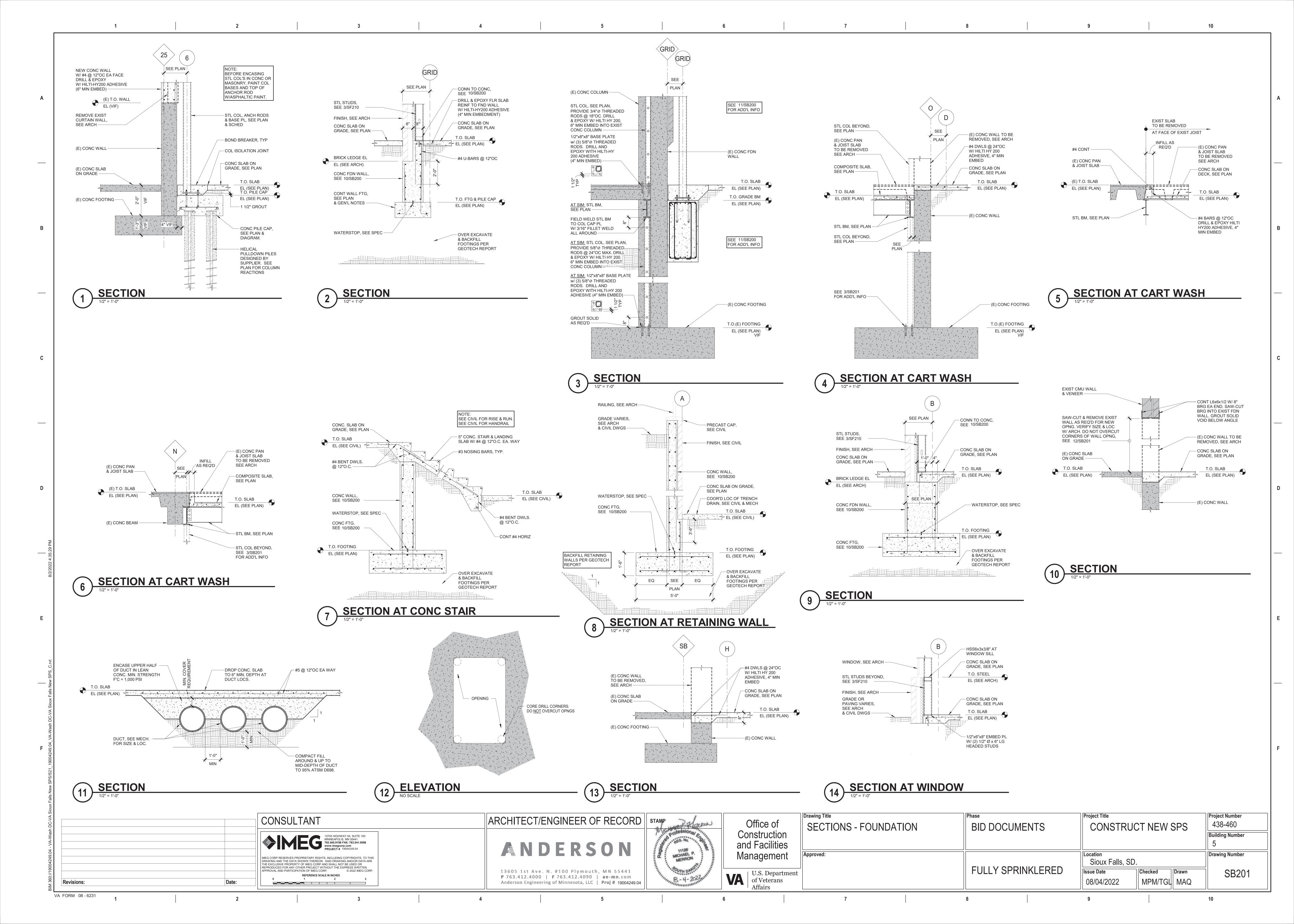


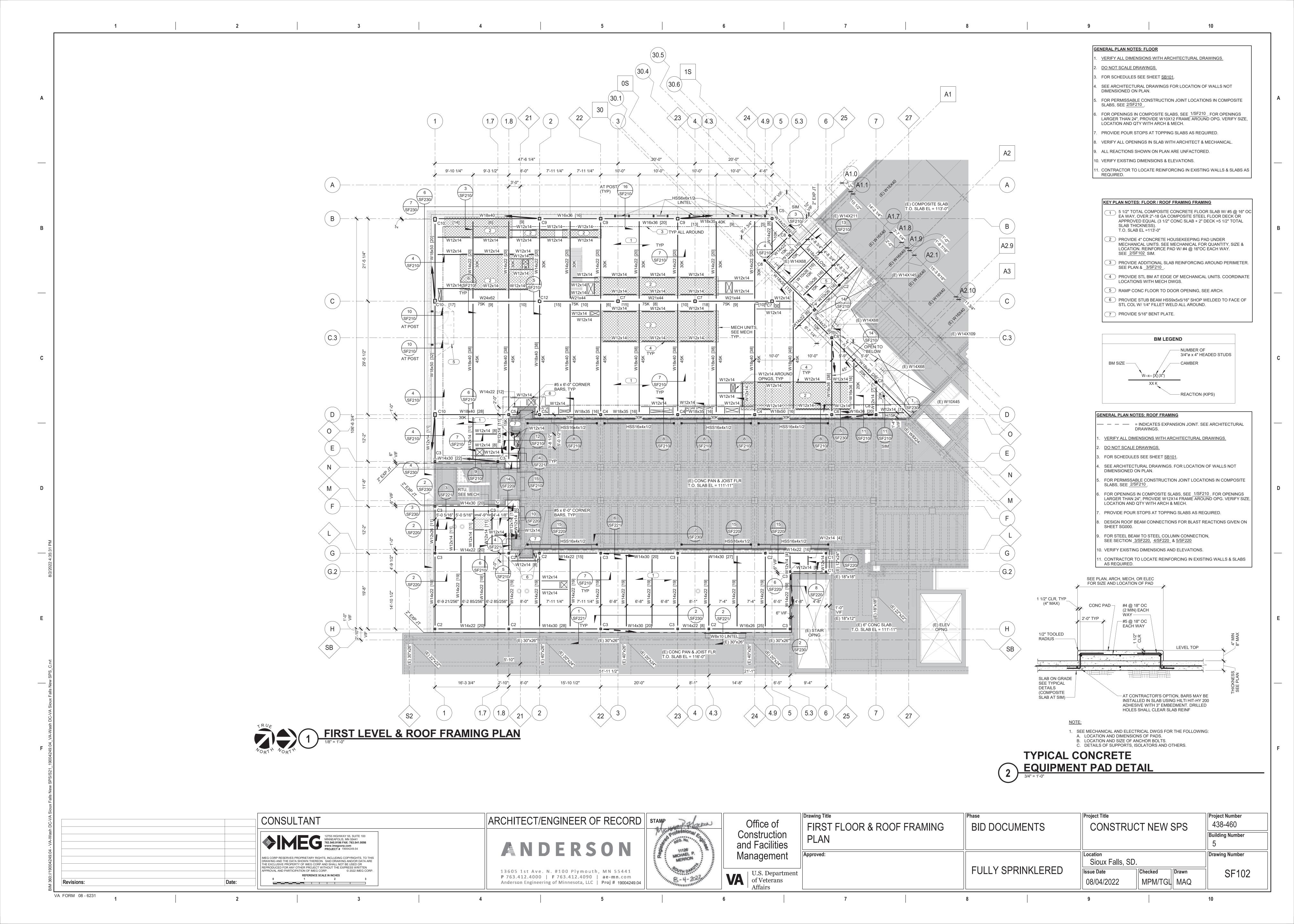
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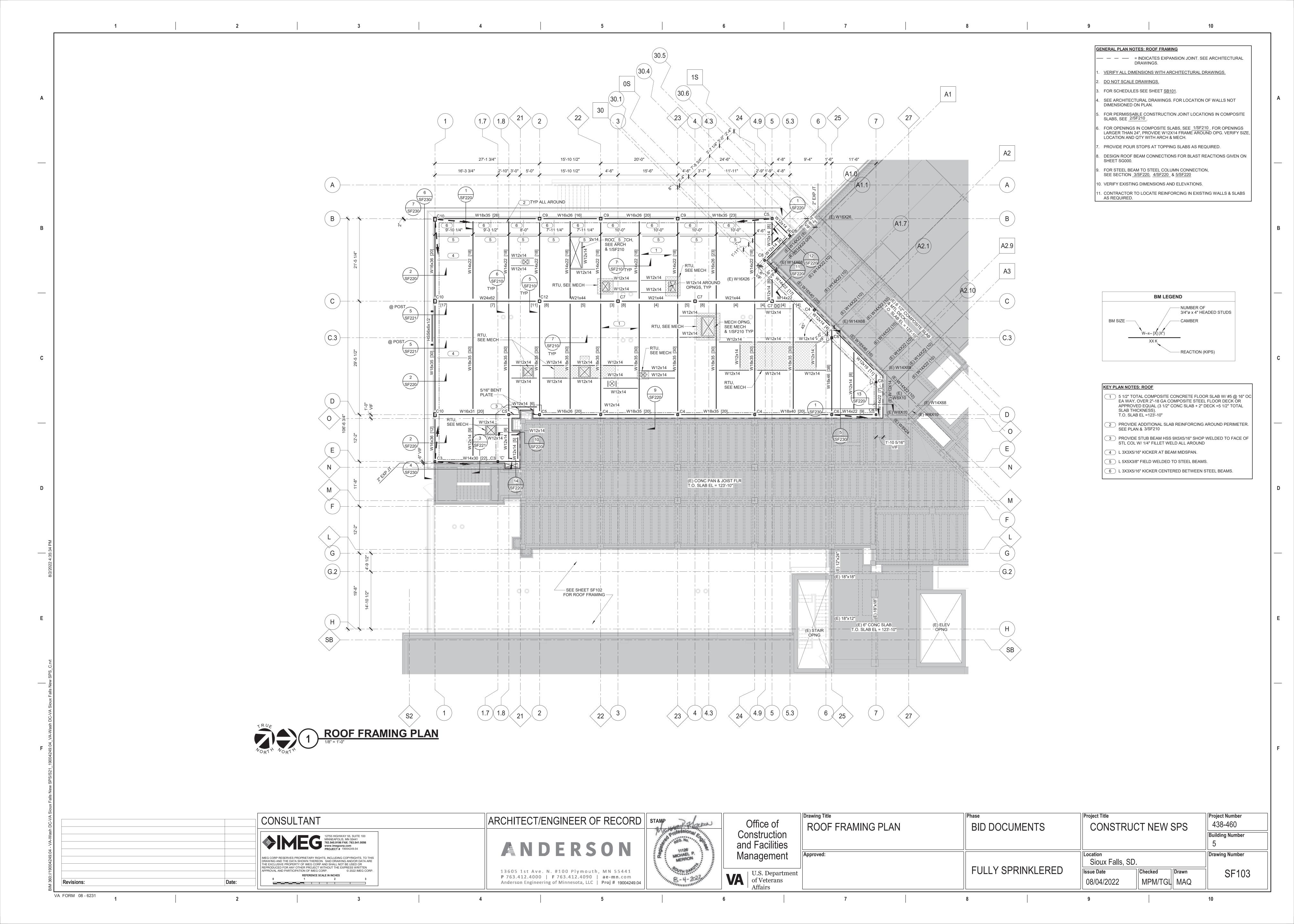
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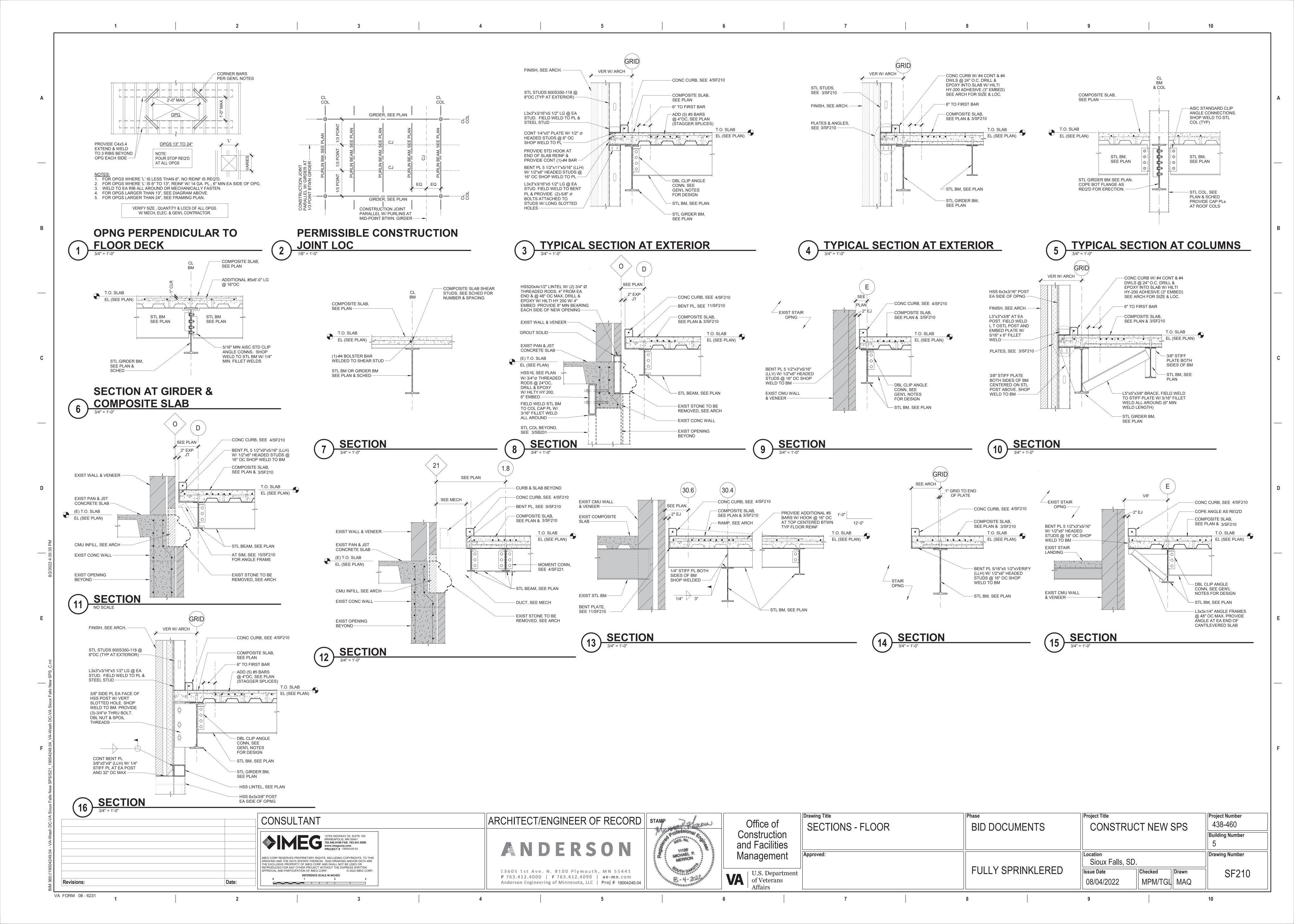
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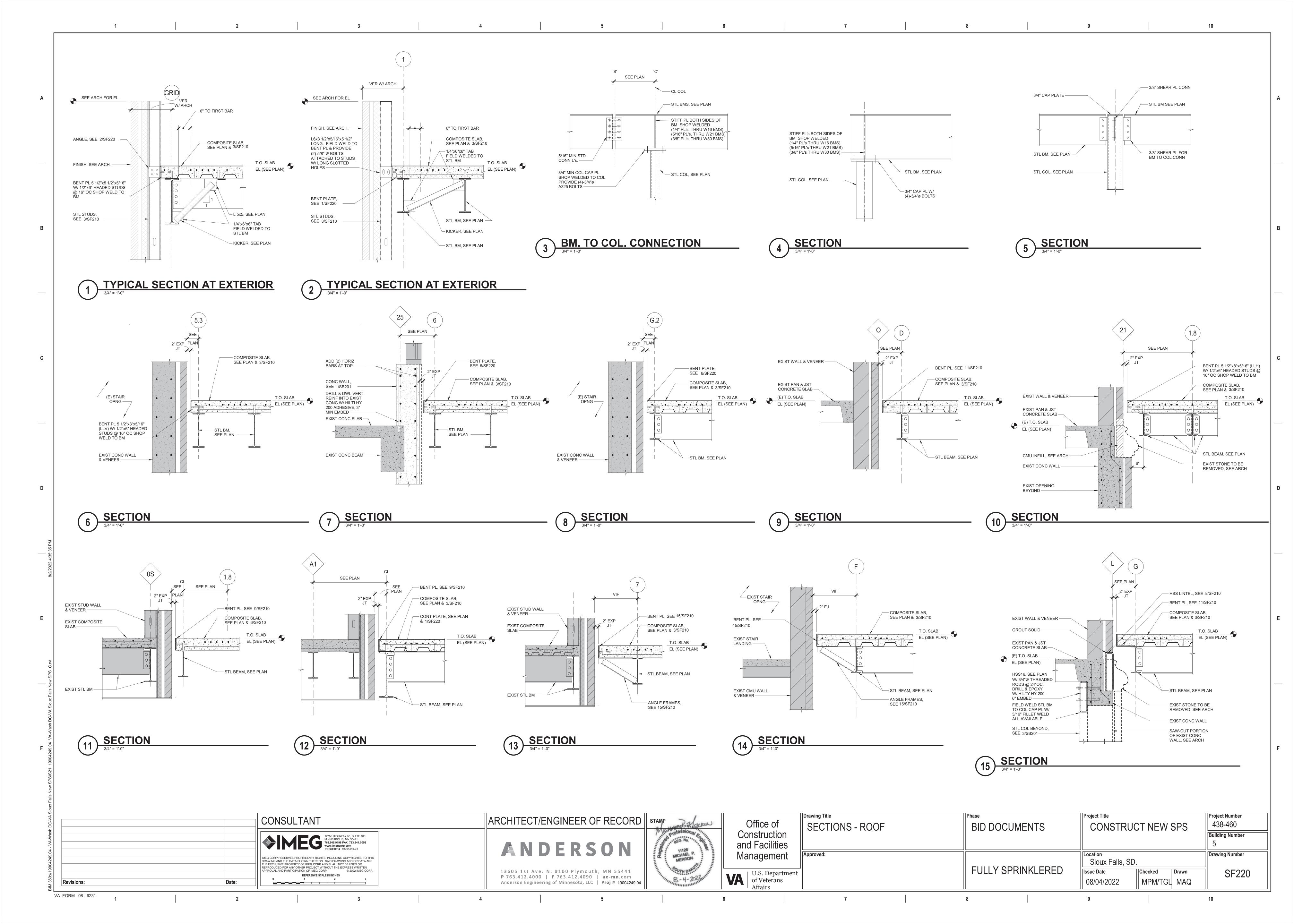


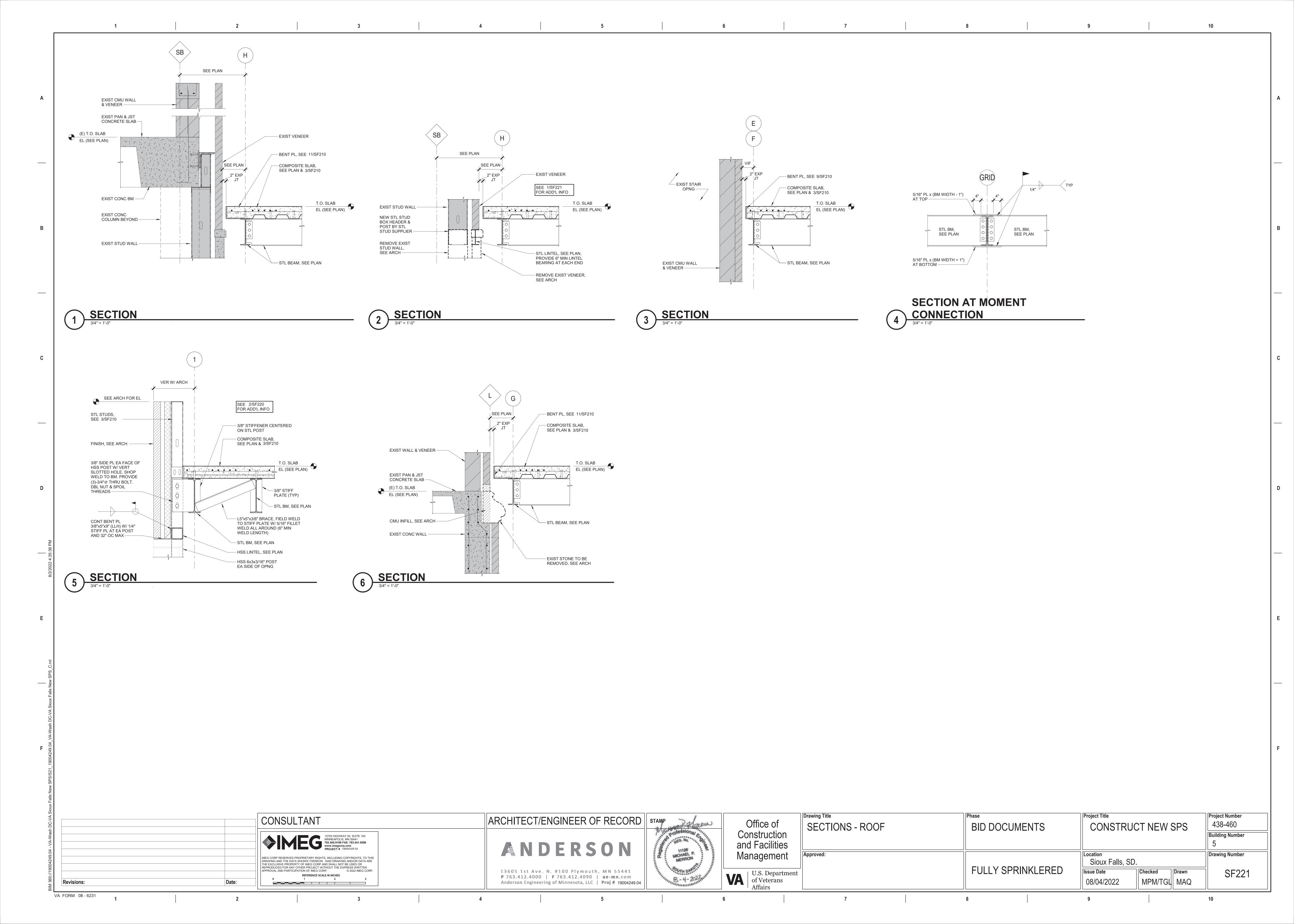


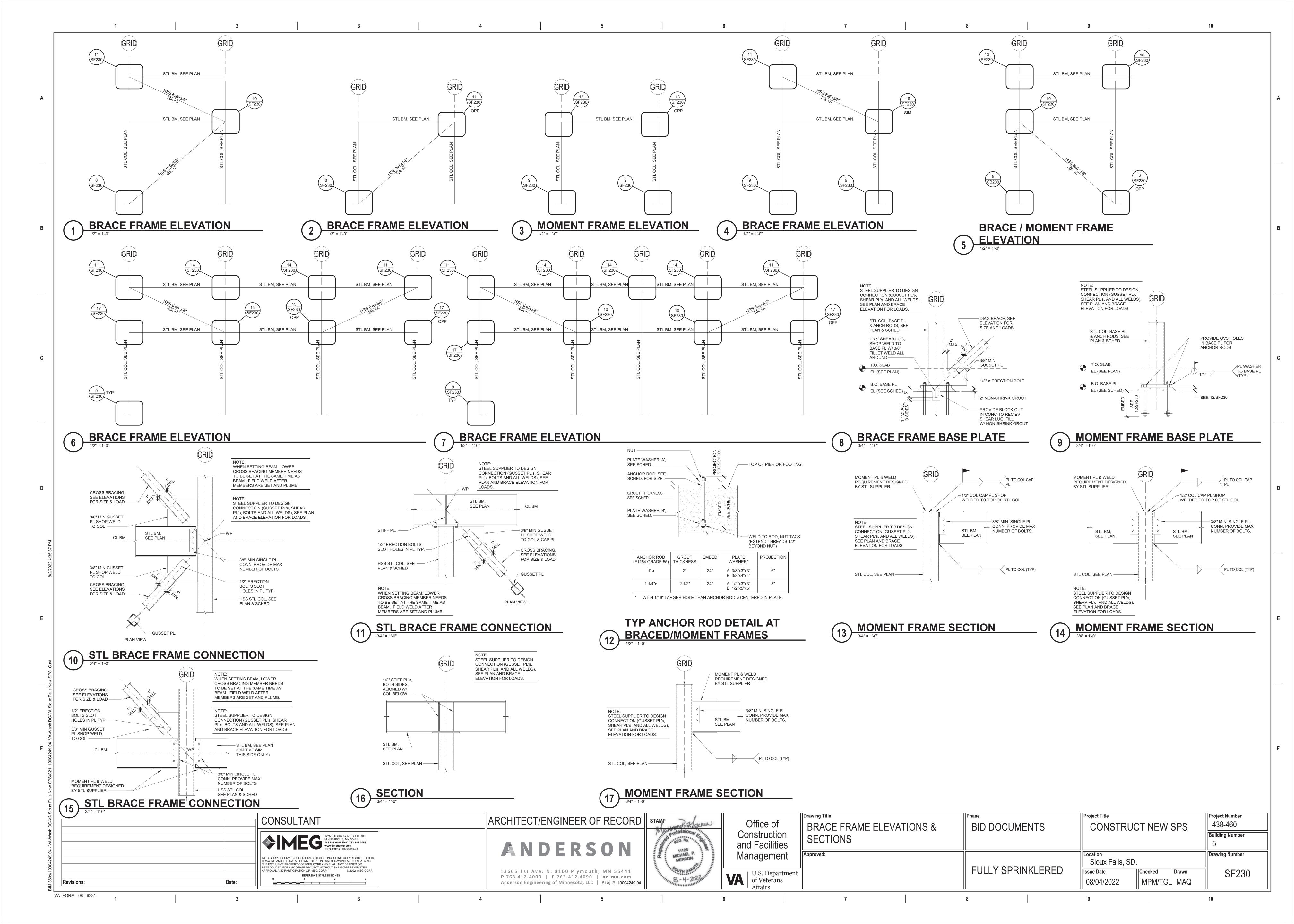






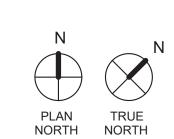






MECH D28 **AUTOCLAVE** GLASSWARE PREP OFFICE D27 D19 **MICROBIOLOGY OFFICE** STORAGE D21 D18 STAIR 9 G81-5 **ELEVATOR LOBBY** G79-5 SEMI-RESTRICTED CORRIDOR G70-5 **GURNEY PARK SCRUB GURNEY**

ASBESTOS CONTAINING MATERIALS LOCATIONS FIRST FLOOR AND BELOW GRADE TUNNEL



ASBESTOS ABATEMENT PHASING:

1. THE ABATEMENT CONTRACTOR SHALL WORK CLOSELY WITH THE GENERAL CONTRACTOR, CONTRACTING OFFICER, OWNER OR OWNER'S REPRESENTATIVE, AND/OR THE VPIH TO COORDINATE REMOVAL OF ACM IN ACCORDANCE WITH PROJECT SCHEDULING, SEQUENCING, AND PHASING REQUIREMENTS. SOME AFTER HOURS AND WEEK-END WORK MAY BE REQUIRED. PHASING IS SUBJECT TO CHANGE TO ACCOMMODATE SITE CONDITIONS AND FACILITY OPERATIONS.

GENERAL NOTES:

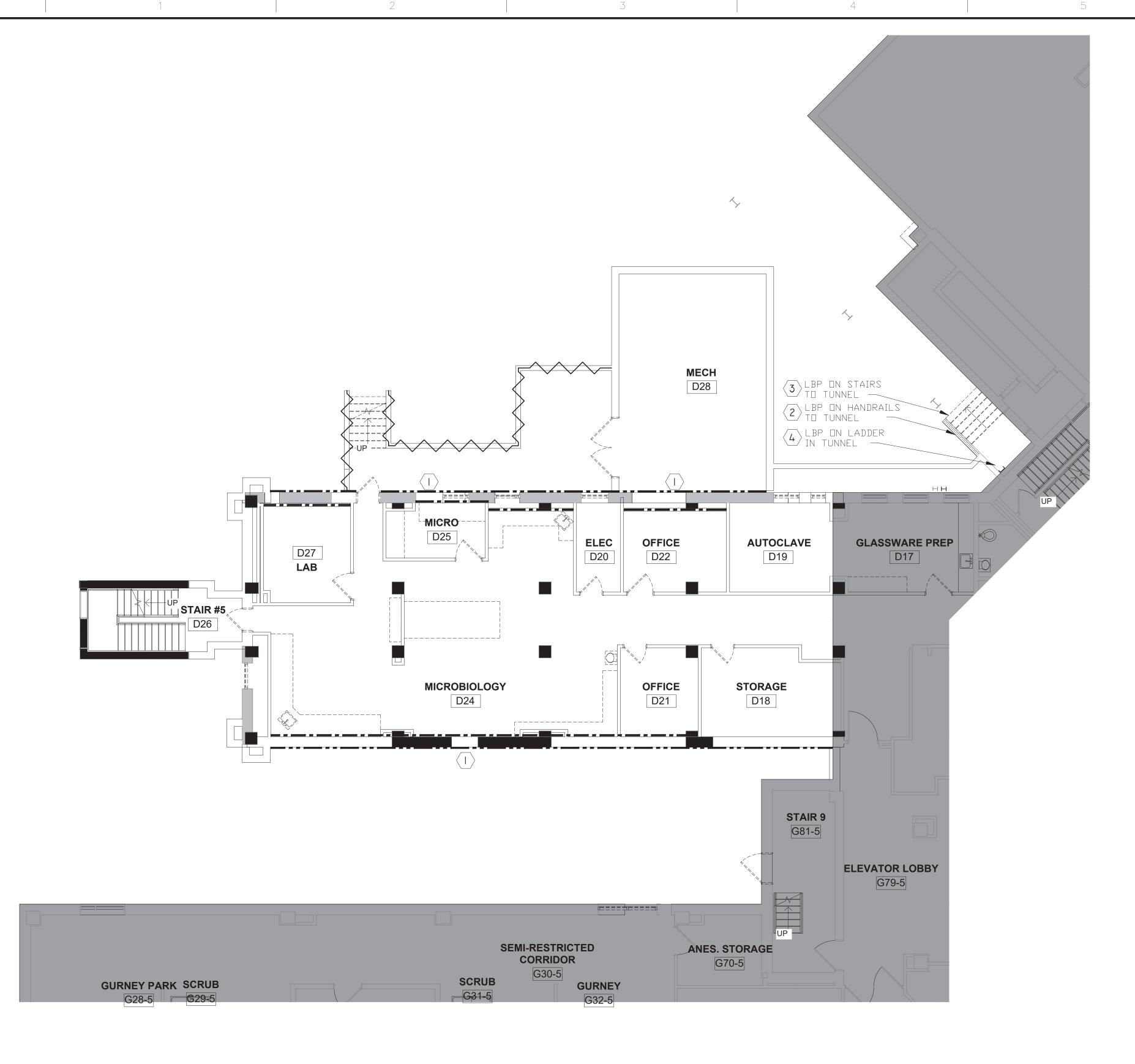
- 1. THESE DRAWINGS ARE DIAGRAMMATIC AND FOR GENERAL IDENTIFICATION OF ASBESTOS-CONTAINING MATERIALS (ACM) AND LEAD-BASED PAINT (LBP) SUBJECT TO REMOVAL OR DISTURBANCE. THEIR ACCURACY IS NOT GUARANTEED. LOCATIONS AND QUANTITIES SHOWN OF ACM AND LBP TO BE REMOVED ARE REPRESENTATIVE BASED ON RECENT AND PREEXISTING SITE SURVEY INFORMATION. THE ABATEMENT CONTRACTOR SHALL BE RESPONSIBLE FOR FIELD VERIFYING ALL MATERIAL LOCATIONS AND REMOVAL QUANTITIES, AND EXISTING SITE CONDITIONS.
- 2. ASBESTOS REMOVAL IS BEING PERFORMED PURSUANT TO RENOVATION OF THE PROJECT AREAS. REMOVE AND DISPOSE OF ALL ACM IN ACCORDANCE WITH APPLICABLE REGULATIONS, PROJECT SPECIFICATIONS, AND THE APPROVED ASBESTOS HAZARD ABATEMENT PLAN (AHAP). IF SUSPECT ACMS ARE ENCOUNTERED DURING CONSTRUCTION AND DEMOLITION THAT ARE NOT IDENTIFIED ON THE ASBESTOS ABATEMENT DRAWINGS, STOP WORK AND CONTACT THE PROJECT MANAGER AND VPIH.
- 3. ALL WORK IS TO BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL REGULATIONS; PROJECT SPECIFICATIONS, THE APPROVED WORK PLAN, AND ACCEPTED INDUSTRY PRACTICE. WHEN REQUIREMENTS OVERLAP OR CONFLICT, THE MOST STRINGENT REQUIREMENT SHALL APPLY. ALL WORK SHALL BE SUBJECT TO INSPECTION BY THE OWNER, THE OWNER'S CONSULTANTS, AND REGULATORY PERSONNEL.
- 4. DEMOLITION OF NON-ACM BUILDING MATERIALS MAY BE REQUIRED TO ACCESS REGULATED MATERIALS, INCLUDING, BUT NOT LIMITED TO, CABINETS, RAISED FLOORING, GYPSUM WALLBOARD, EXPANDED METAL OR WOOD LATH AND PLASTER WALLS AND CEILINGS, WALL FRAMING, CARPET, CERAMIC AND VINYL FLOOR COVERINGS, WOOD, ETC. THE ABATEMENT CONTRACTOR SHALL BE RESPONSIBLE FOR DEMOLITION OF NON-ACM MATERIALS AS NEEDED TO ACCESS REGULATED MATERIALS FOR ABATEMENT, AND FOR COORDINATING THE LIMITS OF DEMOLITION AND ABATEMENT WITH THE GENERAL CONTRACTOR.
- ALL COSTS ASSOCIATED WITH EXPLORATORY DEMOLITION AND DEMOLITION OF NON-ACM MATERIALS NEEDED TO ACCOMPLISH ABATEMENT SHALL BE INCLUDED IN THE ABATEMENT CONTRACTOR'S LUMP SUM PRICE FOR THE PROJECT. NO ADDITIONAL COMPENSATION SHALL BE CONSIDERED FOR THIS WORK.

ASBESTOS NOTES:

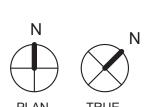
- ESTABLISH REGULATED AREA (MINOR DECONTAMINATION AREA) AND REMOVE 12"X12" VFT AND BLACK MASTIC. ALTHOUGH VFT IS NON-ASBESTOS CONTAINING, THE MASTIC CANNOT BE SEPARATED FROM THE VFT. REMOVE 12X12" FLOOR TILE AND BLACK MASTIC AS REQUIRED BY SECTION 02 82 13.19.
- 2. CONCEALED TSI CONTAINING ACM MAY EXIST IN WALLS, ABOVE CEILING, IN PIPE CHASES, AND WITHIN WALL PENETRATIONS. IF SUSPECT ACMs ARE ENCOUNTERED DURING CONSTRUCTION AND DEMOLITION THAT ARE NOT IDENTIFIED ON THE ASBESTOS ABATEMENT DRAWINGS, STOP WORK AND CONTACT THE PROJECT MANAGER OF VPIH.
- 3. THE PROJECT AREA WAS RECENTLY SURVEYED FOR ACM. REFER TO THE HAZARDOUS BUILDING MATERIALS INSPECTION REPORT BY AMI ENVIRONMENTAL, DATED MAY 22, 2019 REVISED JANUARY 24, 2020 FOR MORE INFORMATION ABOUT ACMs IDENTIFIED IN THE PROJECT
- 4. ASSUME 50% EFFICIENCY WHEN CALCULATING NAM REQUIREMENTS FOR ACHIEVING FOUR (4) AIR CHANGES PER HOUR AND PROVIDED GREATER THAN -0.02" WCG PRESSURE. CONFIGURE AND PLACE NAMS AS NEEDED TO MAXIMIZE AIR MOVEMENT AND PREVENT DEAD AIR SPACE. COORDINATE NEGATIVE AIR DISCHARGE LOCATIONS WITH GENERAL CONTRACTOR, OWNER'S REPRESENTATIVE, AND VPIH, IF NEEDED.

SUMMARY OF LEAD-BASED PAINT MATERIALS							
DESCRIPTION	CONDITION	EST. QTY.	HATCHING				
BLACK MASTIC 12X12 VINYL FLOOR TILE (VFT)		50 FT²					
BLACK MASTIC IN MECHANICAL ROOM		44 FT²					

CONSULTANTS:	ARCHITECT/ENGINEERS:	Drawing Title	Project Title	Project Number	
		4.0050700.00174151110.044750141.0	A CDECTOC CONTAINING	438-460	Office of Construction
		ASBESTOS CONTAINING MATERIALS	ASBESTOS CONTAINING MATERIALS	Building Number	
			IVIATERIALS	5	and Facilities
ENVIRONMENTAL		Approved: Project Director	Location	Drawing Number	Management
AMI ENVIRONMENTAL 8802 SOUTH 135TH STREET,			SIOUX FALLS, SOUTH DAKOTA		
8802 SOUTH 135TH STREET, SUITE 100		SIOUX FALLS	Date Checked Drawn	─ ⊢ ΗΛ1∩1	
OMAHA, NEBRASKA, 68138		VAMC			(22)
Revisions: Date PH: (402) 397-3313			08/04/2022 WHC MET		



LEAD-BASED PAINT LOCATIONS
FIRST FLOOR AND BELOW GRADE TUNNEL



LEAD NOTES:

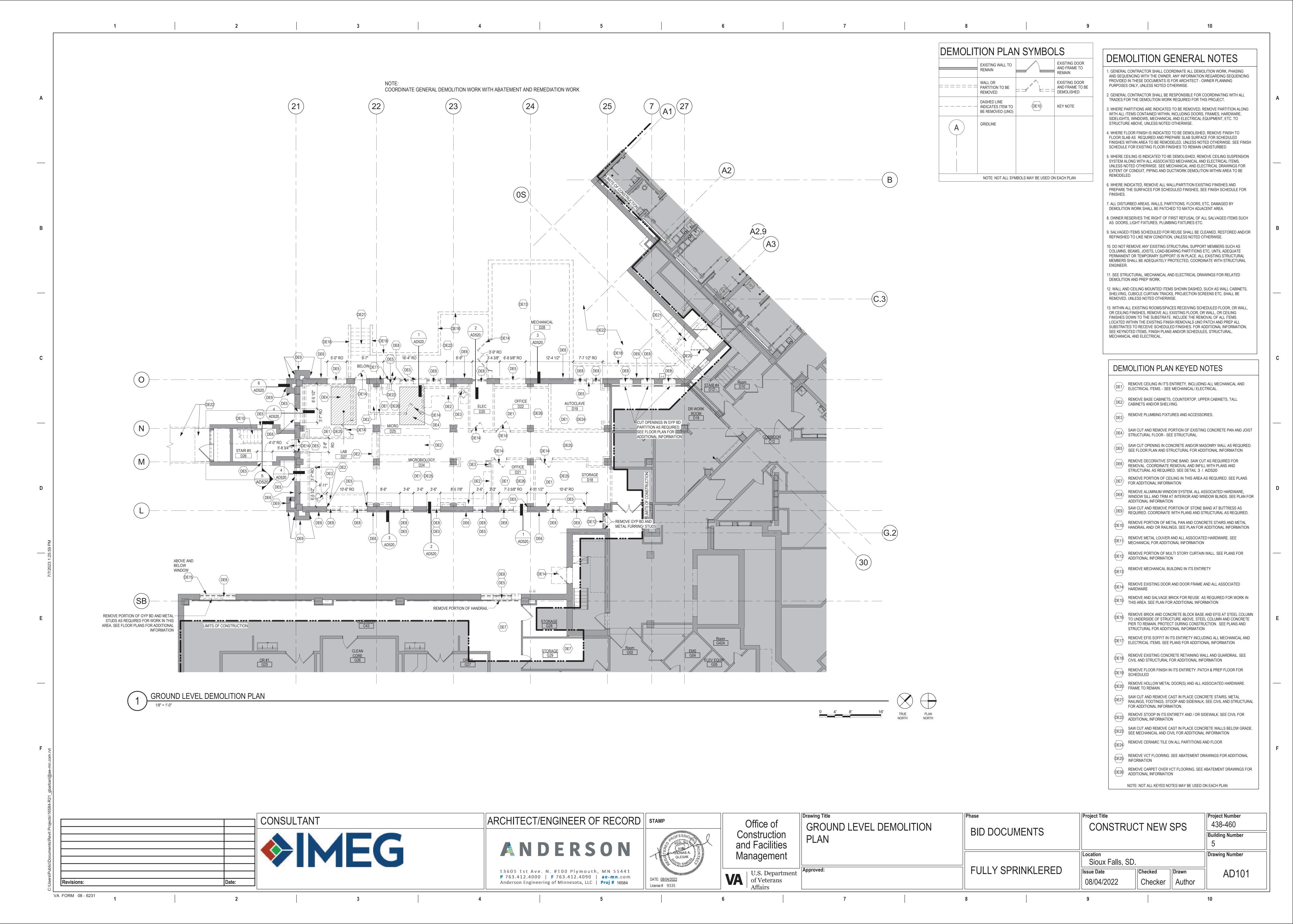
- 1. LBP IS KNOWN TO EXIST ON MATERIALS, COMPONENTS, AND SURFACES THAT MAY BE DISTURBED, PENETRATED, REFINISHED, OR DEMOLISHED. PERFORM DEMOLITION OF MATERIALS AND COMPONENTS WITH LBP AND/OR PCL IN ACCORDANCE WITH APPLICABLE REGULATIONS, SECTION 02 83 33.13, LEAD-BASED PAINT REMOVAL AND DISPOSAL AND THE APPROVED WORK PLAN.
- 2. CONCEALED LBP MAY BE PRESENT ON SURFACES BEHIND WALLS AND MAY BE IMPACTED FOR PENETRATIONS, OR WALL DEMOLITION. LBP DUST MUST BE CONTROLLED ACCORDING THE 29 CFR 1926.62. PERFORM CLEANUP AND DISPOSAL OF LBP DUST AND DEBRIS IN ACCORDANCE WITH SECTION 02 83 33.13, LEAD-BASED PAINT REMOVAL AND DISPOSAL.
- 3. LEAD-BASED PAINT (LBP) ARE PAINTS THAT CONTAIN LEAD
 ≥1.0 mg/m² or ≥0.5 PERCENT BY WEIGHT. PAINT CONTAINING
 LEAD (PCL) IS PAINT WITH A DETECTABLE LEVEL OF LEAD.
 LBP AND PCL ARE KNOWN TO EXIST ON MATERIALS,
 COMPONENTS, AND SURFACES THAT MAY BE DISTURBED,
 PENETRATED, REFINISHED, OR DEMOLISHED. PERFORM
 DEMOLITION OF MATERIALS AND COMPONENTS WITH LBP
 AND/OR PCL IN ACCORDANCE WITH APPLICABLE
 REGULATIONS AND THE APPROVED WORK PLAN.
- FLAKING AND PEELING LBP AND/OR PCL ON SURFACES TO REMAIN SHALL BE REMOVED AND STABILIZED USING METHODS IN ACCORDANCE WITH SECTION 02 83 33.13, LEAD-BASED PAINT REMOVAL AND DISPOSAL.
- REFER TO THE HAZARDOUS BUILDING MATERIALS INSPECTION REPORT BY AMI ENVIRONMENTAL, DATED MAY 22, 2019, REVISED JANUARY 24, 2020 FOR INFORMATION CONCERNING THE PRESENCE OF LBP AND PCL IN THE PROJECT AREAS.

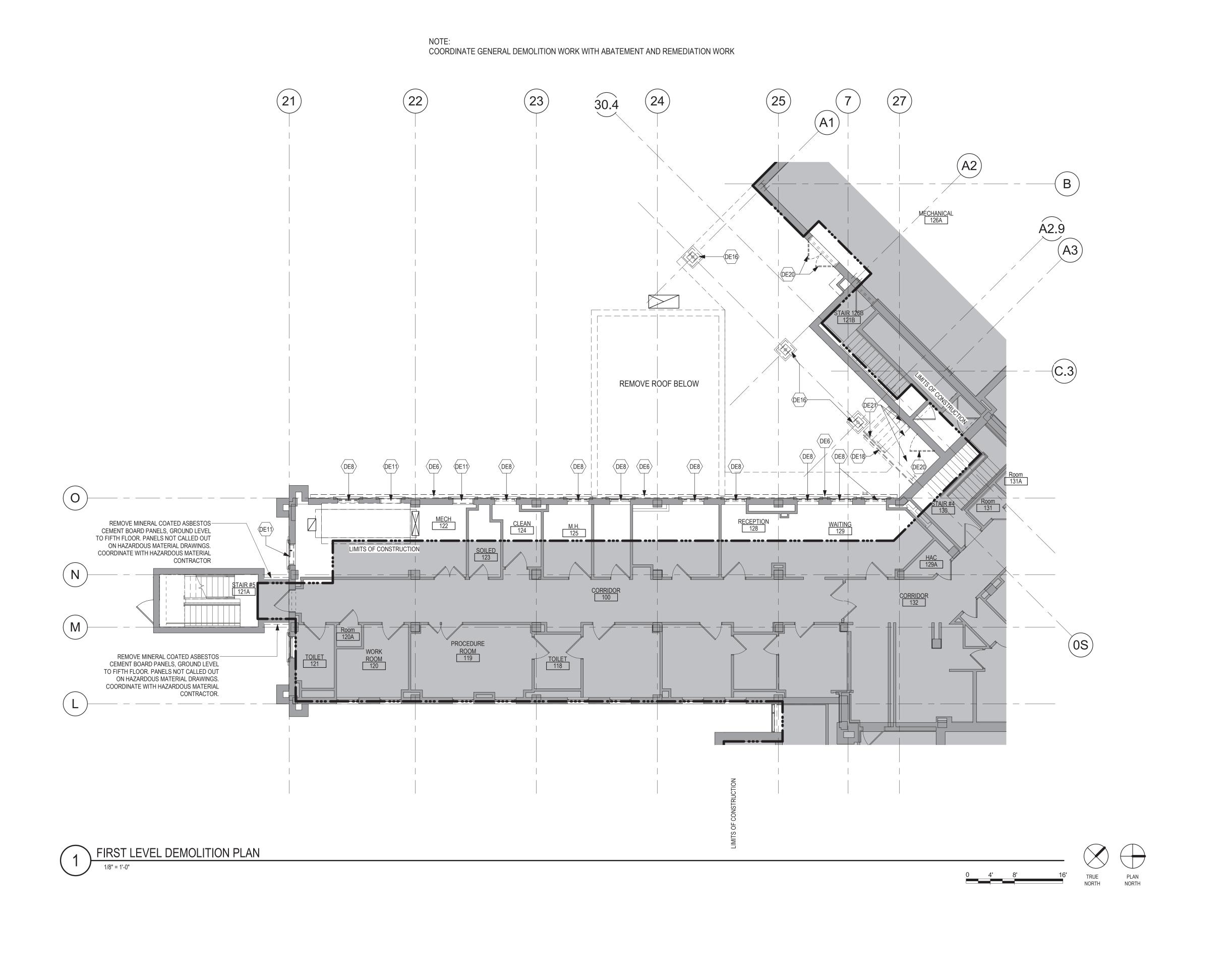
DESCRIPTION	CONDITION	EST. QTY.	HATCHING OR KEYNOTE
LEAD-BASED PAINT (LBP) ON EXTERIOR HAND RAILS	FAIR	150 LF	-^\\
LBP ON EXTERIOR GRATES	POOR	15 FT²	
LBP ON EXTERIOR WINDOW TOP PLATES	POOR	44 FT²	
LBP ON INTERIOR WINDOW SILLS	FAIR	50 FT²	
LBP ON HANDRAIL TO TUNNEL	FAIR	25 LF	<u></u>
LBP ON STAIRS IN TUNNEL	FAIR	100 FT²	(3)
LBP ON LADDER IN TUNNEL	FAIR	1 EACH	<u>\langle</u>

		Drawing Title	Project Title	Project Number	
CONSULTANTS: AMI	ARCHITECT/ENGINEERS:	LEAD CONTAINING MATERIAL	SIOUX FALLS VAMC NEW SPS ADDITION	438-460 Building Number	Office of Construction and Facilities Management
Revisions: ENVIRONMENTAL AMI ENVIRONMENTAL 8802 SOUTH 135TH STREET, SUITE 100 OMAHA, NEBRASKA, 68138 PH: (402) 397-3313		Approved: Project Director SIOUX FALLS VAMC	Date 08/04/2022 Checked Drawn MET	Drawing Number HA102	Management

 1
 3

 4





DEMOLITION PLAN SYMBOLS EXISTING DOOR EXISTING WALL TO AND FRAME TO EXISTING DOOR ====== AND FRAME TO BE PARTITION TO BE DEMOLISHED REMOVED DASHED LINE 〈DE10〉 KEY NOTE INDICATES ITEM TO BE REMOVED (UNO)

NOTE: NOT ALL SYMBOLS MAY BE USED ON EACH PLAN

DEMOLITION GENERAL NOTES

1. GENERAL CONTRACTOR SHALL COORDINATE ALL DEMOLITION WORK, PHASING AND SEQUENCING WITH THE OWNER. ANY INFORMATION REGARDING SEQUENCING PROVIDED IN THESE DOCUMENTS IS FOR ARCHITECT - OWNER PLANNING PURPOSES ONLY, UNLESS NOTED OTHERWISE.

2. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING WITH ALL

STRUCTURE ABOVE, UNLESS NOTED OTHERWISE.

TRADES FOR THE DEMOLITION WORK REQUIRED FOR THIS PROJECT. 3. WHERE PARTITIONS ARE INDICATED TO BE REMOVED, REMOVE PARTITION ALONG WITH ALL ITEMS CONTAINED WITHIN, INCLUDING DOORS, FRAMES, HARDWARE, SIDELIGHTS, WINDOWS, MECHANICAL AND ELECTRICAL EQUIPMENT, ETC. TO

4. WHERE FLOOR FINISH IS INDICATED TO BE DEMOLISHED, REMOVE FINISH TO FLOOR SLAB AS REQUIRED AND PREPARE SLAB SURFACE FOR SCHEDULED FINISHES WITHIN AREA TO BE REMODELED, UNLESS NOTED OTHERWISE. SEE FINISH SCHEDULE FOR EXISTING FLOOR FINISHES TO REMAIN UNDISTURBED.

5. WHERE CEILING IS INDICATED TO BE DEMOLISHED, REMOVE CEILING SUSPENSION SYSTEM ALONG WITH ALL ASSOCIATED MECHANICAL AND ELECTRICAL ITEMS, UNLESS NOTED OTHERWISE. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR EXTENT OF CONDUIT, PIPING AND DUCTWORK DEMOLITION WITHIN AREA TO BE REMODELED.

6. WHERE INDICATED, REMOVE ALL WALL/PARTITION EXISTING FINISHES AND PREPARE THE SURFACES FOR SCHEDULED FINISHES, SEE FINISH SCHEDULE FOR

7. ALL DISTURBED AREAS, WALLS, PARTITIONS, FLOORS, ETC, DAMAGED BY DEMOLITION WORK SHALL BE PATCHED TO MATCH ADJACENT AREA.

8. OWNER RESERVES THE RIGHT OF FIRST REFUSAL OF ALL SALVAGED ITEMS SUCH AS: DOORS, LIGHT FIXTURES, PLUMBING FIXTURES ETC.

9. SALVAGED ITEMS SCHEDULED FOR REUSE SHALL BE CLEANED, RESTORED AND/OR

REFINISHED TO LIKE NEW CONDITION, UNLESS NOTED OTHERWISE.

10. DO NOT REMOVE ANY EXISTING STRUCTURAL SUPPORT MEMBERS SUCH AS COLUMNS, BEAMS, JOISTS, LOAD-BEARING PARTITIONS ETC, UNTIL ADEQUATE PERMANENT OR TEMPORARY SUPPORT IS IN PLACE. ALL EXISTING STRUCTURAL MEMBERS SHALL BE ADEQUATELY PROTECTED, COORDINATE WITH STRUCTURAL

11. SEE STRUCTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR RELATED DEMOLITION AND PREP WORK.

12. WALL AND CEILING MOUNTED ITEMS SHOWN DASHED, SUCH AS WALL CABINETS, SHELVING, CUBICLE CURTAIN TRACKS, PROJECTION SCREENS ETC, SHALL BE REMOVED, UNLESS NOTED OTHERWISE.

13. WITHIN ALL EXISTING ROOMS/SPACES RECEIVING SCHEDULED FLOOR, OR WALL, OR CEILING FINISHES, REMOVE ALL EXISTING FLOOR, OR WALL, OR CEILING FINISHES DOWN TO THE SUBSTRATE. INCLUDE THE REMOVAL OF ALL ITEMS LOCATED WITHIN THE EXISTING FINISH REMOVALS UNO PATCH AND PREP ALL SUBSTRATES TO RECEIVE SCHEDULED FINISHES. FOR ADDITIONAL INFORMATION, SEE KEYNOTED ITEMS, FINISH PLANS AND/OR SCHEDULES, STRUCTURAL, MECHANICAL AND ELECTRICAL.

DEMOLITION PLAN KEYED NOTES

- REMOVE CEILING IN IT'S ENTIRETY, INCLUDING ALL MECHANICAL AND (DE1) ELECTRICAL ITEMS. - SEE MECHANICAL/ ELECTRICAL.
- REMOVE BASE CABINETS, COUNTERTOP, UPPER CABINETS, TALL CABINETS AND/OR SHELVING.
- DE3 REMOVE PLUMBING FIXTURES AND ACCESSORIES.

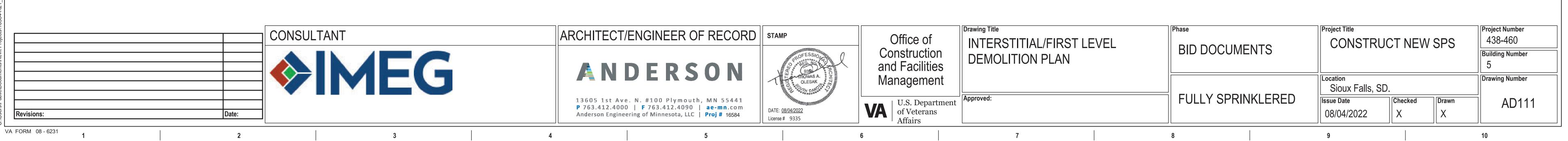
ADDITIONAL INFORMATION

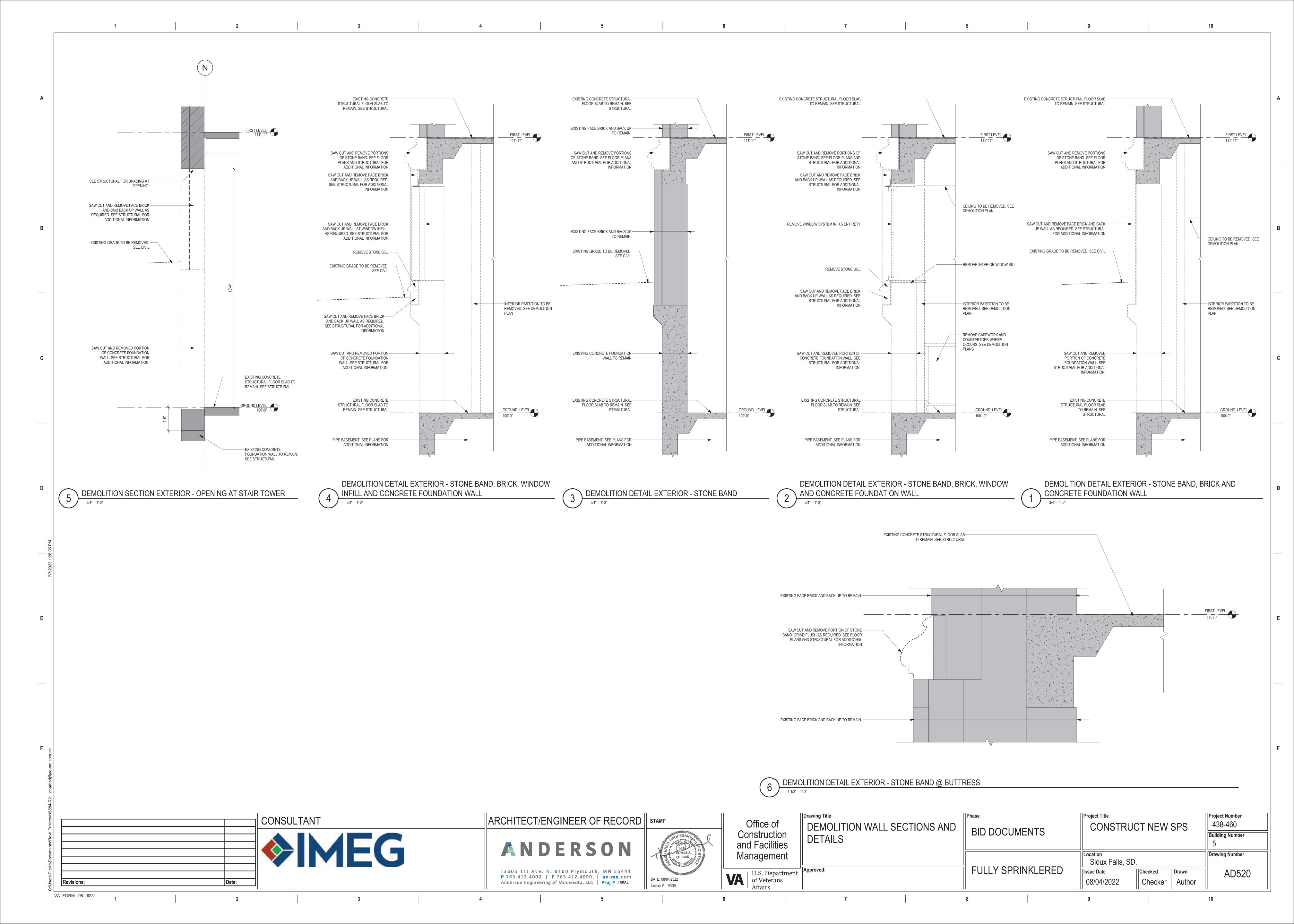
- SAW CUT AND REMOVE PORTION OF EXISTING CONCRETE PAN AND JOIST STRUCTURAL FLOOR SEE STRUCTURAL.
- SAW CUT OPENING IN CONCRETE AND/OR MASONRY WALL AS REQUIRED. SEE FLOOR PLAN AND STRUCTURAL FOR ADDITIONAL INFORMATION
- REMOVE DECORATIVE STONE BAND. SAW CUT AS REQUIRED FOR REMOVAL. COORDINATE REMOVAL AND INFILL WITH PLANS AND
- STRUCTURAL AS REQUIRED. SEE DETAIL 3 / AD520 REMOVE PORTION OF CEILING IN THIS AREA AS REQUIRED. SEE PLANS FOR ADDITIONAL INFORMATION
- REMOVE ALUMINUM WINDOW SYSTEM, ALL ASSOCIATED HARDWARE, WINDOW SILL AND TRIM AT INTERIOR AND WINDOW BLINDS. SEE PLAN FOR
- SAW CUT AND REMOVE PORTION OF STONE BAND AT BUTTRESS AS REQUIRED. COORDINATE WITH PLANS AND STRUCTURAL AS REQUIRED.
- DE10 REMOVE PORTION OF METAL PAN AND CONCRETE STAIRS AND METAL HANDRAIL AND/ OR RAILINGS. SEE PLAN FOR ADDITIONAL INFORMATION
- REMOVE METAL LOUVER AND ALL ASSOCIATED HARDWARE. SEE MECHANICAL FOR ADDITIONAL INFORMATION
- REMOVE PORTION OF MULTI STORY CURTAIN WALL. SEE PLANS FOR ADDITIONAL INFORMATION
- DE13 REMOVE MECHANICAL BUILDING IN ITS ENTIRETY

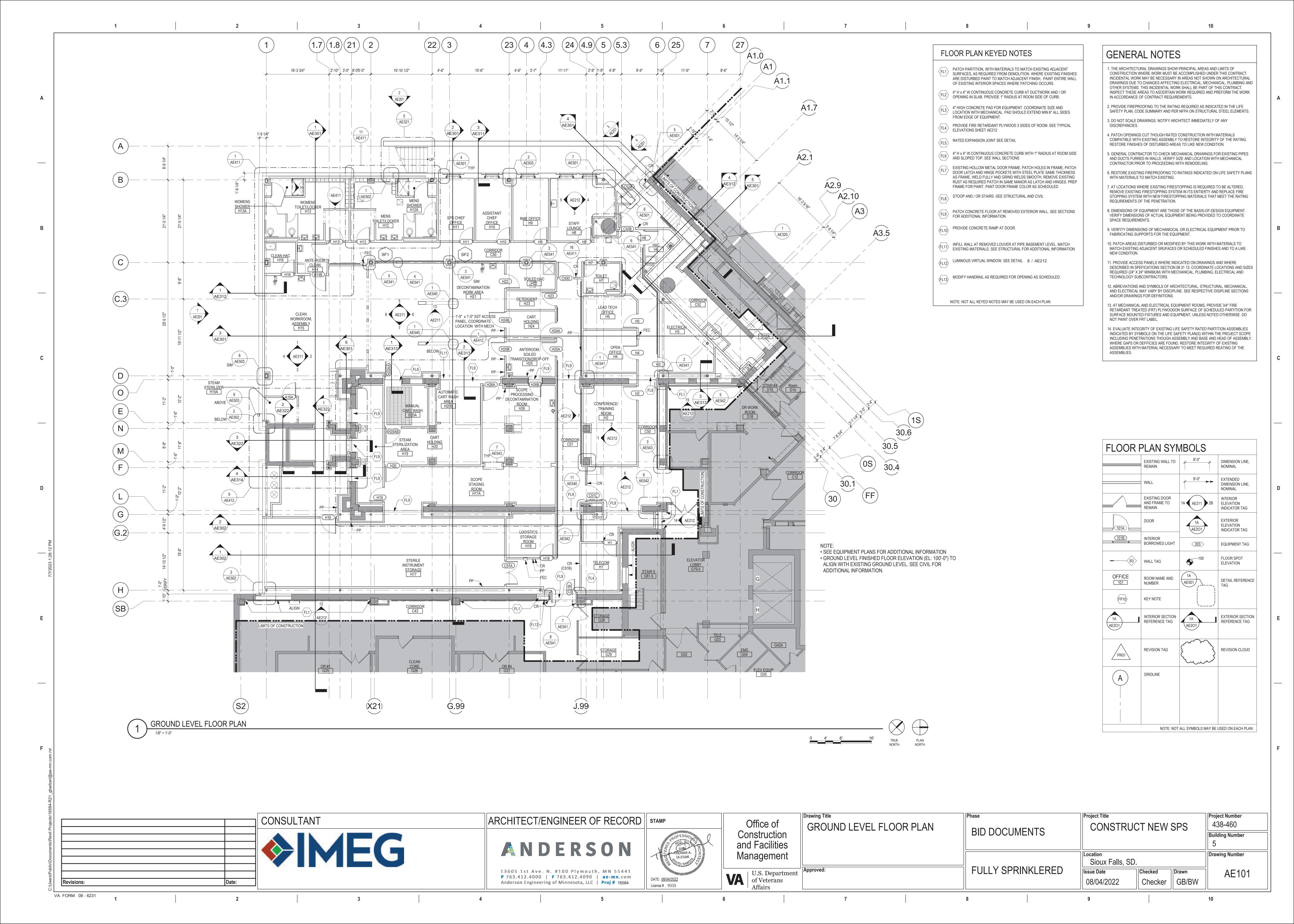
STRUCTURAL FOR ADDITIONAL INFORMATION

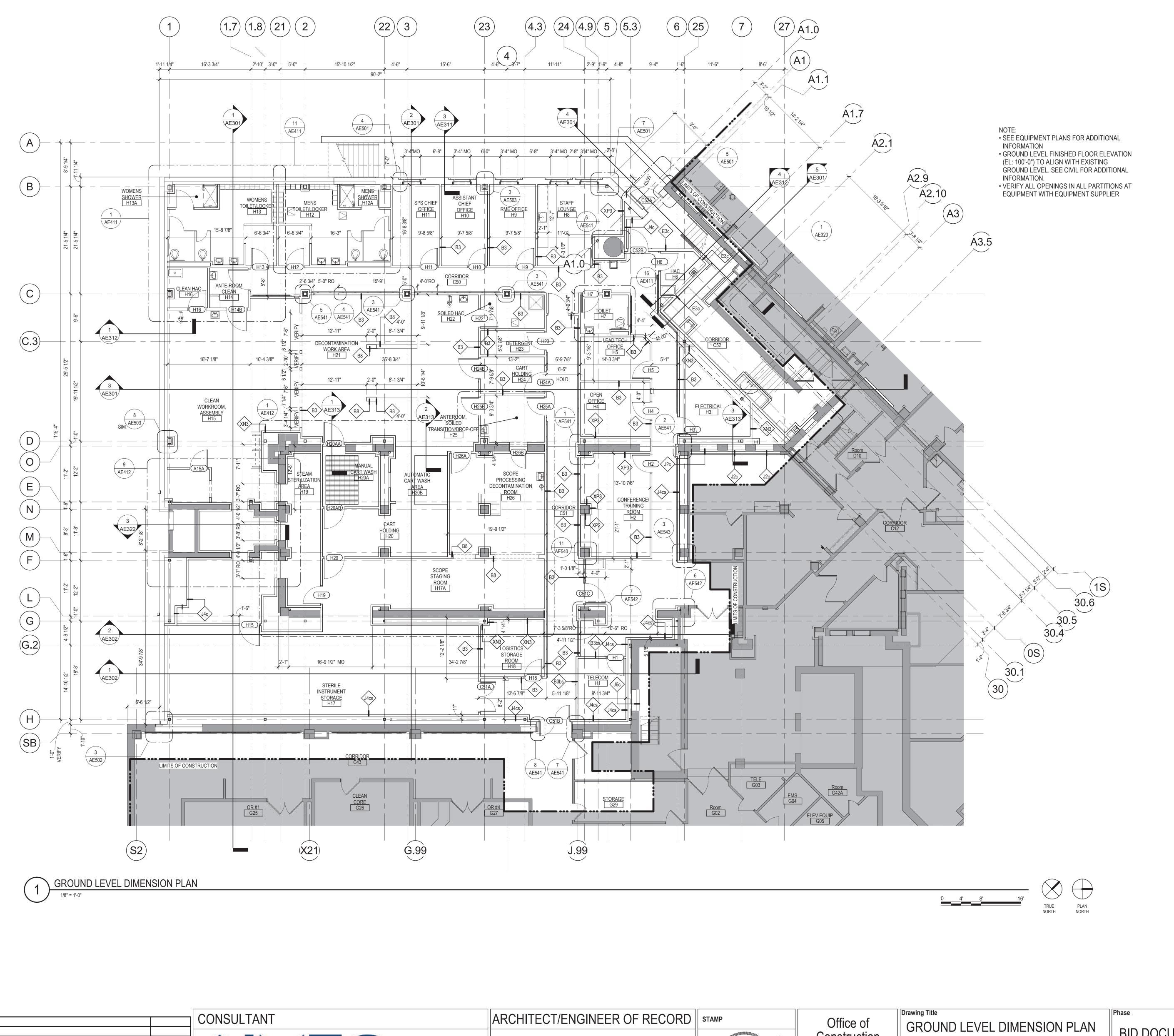
- DE14 REMOVE EXISTING DOOR AND DOOR FRAME AND ALL ASSOCIATED HARDWARE
- REMOVE AND SALVAGE BRICK FOR REUSE AS REQUIRED FOR WORK IN THIS AREA. SEE PLAN FOR ADDITIONAL INFORMATION
- REMOVE BRICK AND CONCRETE BLOCK BASE AND EFIS AT STEEL COLUMN TO UNDERSIDE OF STRUCTURE ABOVE. STEEL COLUMN AND CONCRETE PIER TO REMAIN, PROTECT DURING CONSTRUCTION. SEE PLANS AND
- REMOVE EFIS SOFFIT IN ITS ENTIRETY INCLUDING ALL MECHANICAL AND ELECTRICAL ITEMS. SEE PLANS FOR ADDITIONAL INFORMATION
- REMOVE EXISTING CONCRETE RETAINING WALL AND GUARDRAIL. SEE CIVIL AND STRUCTURAL FOR ADDITIONAL INFORMATION
- REMOVE FLOOR FINISH IN ITS ENTIRETY. PATCH & PREP FLOOR FOR SCHEDULED
- REMOVE HOLLOW METAL DOOR(S) AND ALL ASSOCIATED HARDWARE. FRAME TO REMAIN.
- SAW CUT AND REMOVE CAST IN PLACE CONCRETE STAIRS, METAL RAILINGS, FOOTINGS, STOOP AND SIDEWALK, SEE CIVIL AND STRUCTURAL
- FOR ADDITIONAL INFORMATION. REMOVE STOOP IN ITS ENTIRETY AND / OR SIDEWALK. SEE CIVIL FOR ADDITIONAL INFORMATION
- SAW CUT AND REMOVE CAST IN PLACE CONCRETE WALLS BELOW GRADE.
 SEE MECHANICAL AND CIVIL FOR ADDITIONAL INFORMATION
- REMOVE CERAMIC TILE ON ALL PARTITIONS AND FLOOR
- DE25 REMOVE VCT FLOORING. SEE ABATEMENT DRAWINGS FOR ADDITIONAL INFORMATION
- REMOVE CARPET OVER VCT FLOORING. SEE ABATEMENT DRAWINGS FOR DE26 ADDITIONAL INFORMATION

NOTE: NOT ALL KEYED NOTES MAY BE USED ON EACH PLAN









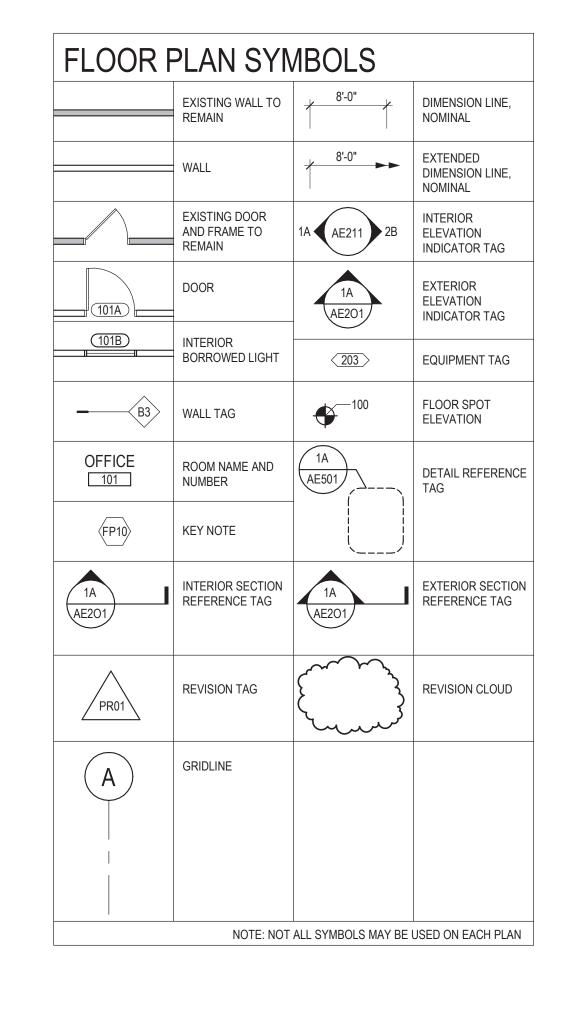
GENERAL NOTES

- THE ARCHITECTURAL DRAWINGS SHOW PRINCIPAL AREAS AND LIMITS OF CONSTRUCTION WHERE WORK MUST BE ACCOMPLISHED UNDER THIS CONTRACT. INCIDENTAL WORK MAY BE NECESSARY IN AREAS NOT SHOWN ON ARCHITECTURAL DRAWINGS DUE TO CHANGES AFFECTING ELECTRICAL, MECHANICAL, PLUMBING AND OTHER SYSTEMS. THIS INCIDENTAL WORK SHALL BE PART OF THIS CONTRACT. INSPECT THESE AREAS TO ASCERTAIN WORK REQUIRED AND PREFORM THE WORK IN ACCORDANCE OF CONTRACT REQUIREMENTS.
- 2. PROVIDE FIREPROOFING TO THE RATING REQUIRED AS INDICATED IN THE LIFE SAFETY PLAN, CODE SUMMARY AND PER NFPA ON STRUCTURAL STEEL ELEMENTS.
- 3. DO NOT SCALE DRAWINGS. NOTIFY ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES.
- 4. PATCH OPENINGS CUT THOUGH RATED CONSTRUCTION WITH MATERIALS COMPATIBLE WITH EXISTING ASSEMBLY TO RESTORE INTEGRITY OF THE RATING. RESTORE FINISHES OF DISTURBED AREAS TO LIKE NEW CONDITION.
- 5. GENERAL CONTRACTOR TO CHECK MECHANICAL DRAWINGS FOR EXISTING PIPES
- AND DUCTS FURRED IN WALLS. VERIFY SIZE AND LOCATION WITH MECHANICAL CONTRACTOR PRIOR TO PROCEEDING WITH REMODELING.
- 6. RESTORE EXISTING FIREPROOFING TO RATINGS INDICATED ON LIFE SAFETY PLANS WITH MATERIALS TO MATCH EXISTING.
- 7. AT LOCATIONS WHERE EXISTING FIRESTOPPING IS REQUIRED TO BE ALTERED, REMOVE EXISTING FIRESTOPPING SYSTEM IN ITS ENTIERTY AND REPLACE FIRE STOPPING SYSTEM WITH NEW FIRESTOPPING MATERIALS THAT MEET THE RATING
- REQUIREMENTS OF THE PENETRATION.

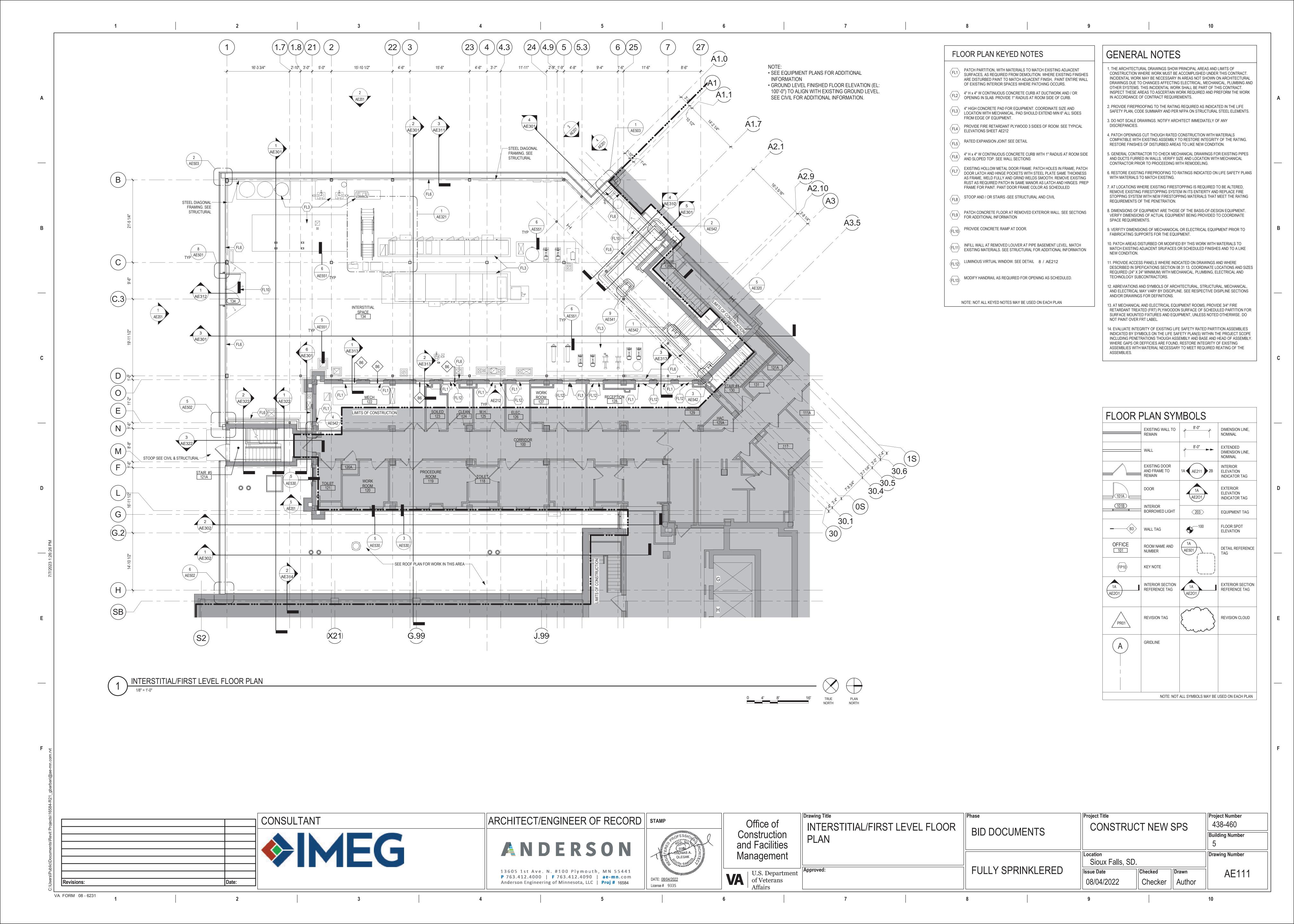
 8. DIMENSIONS OF EQUIPMENT ARE THOSE OF THE BASIS-OF-DESIGN EQUIPMENT. VERIFY DIMENSIONS OF ACTUAL EQUIPMENT BEING PROVIDED TO COORDINATE
- SPACE REQUIREMENTS.

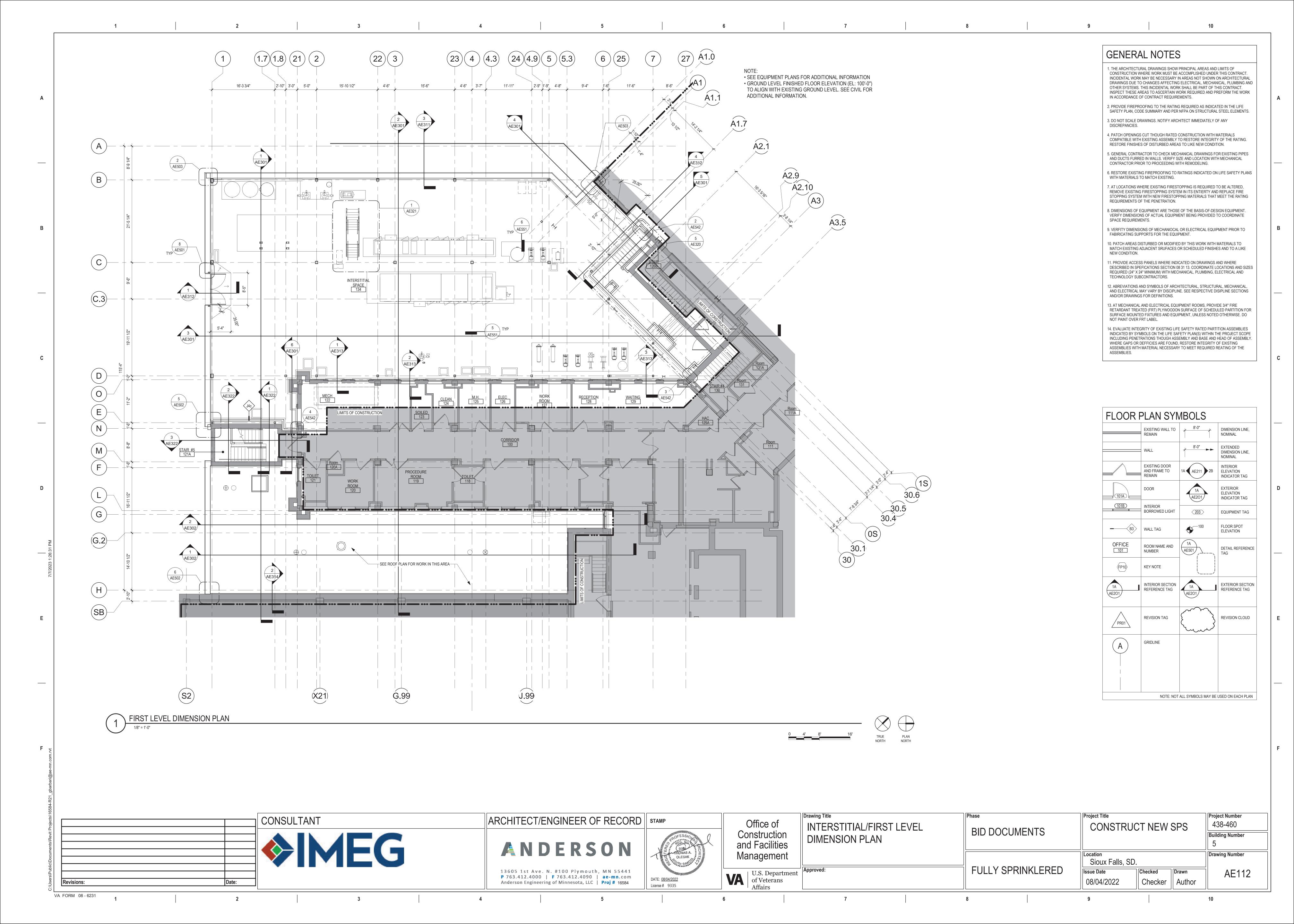
 9. VERFITY DIMENSIONS OF MECHANIOCAL OR ELECTRICAL EQUIPMENT PRIOR TO
- FABIRICATING SUPPORTS FOR THE EQUIPMENT.

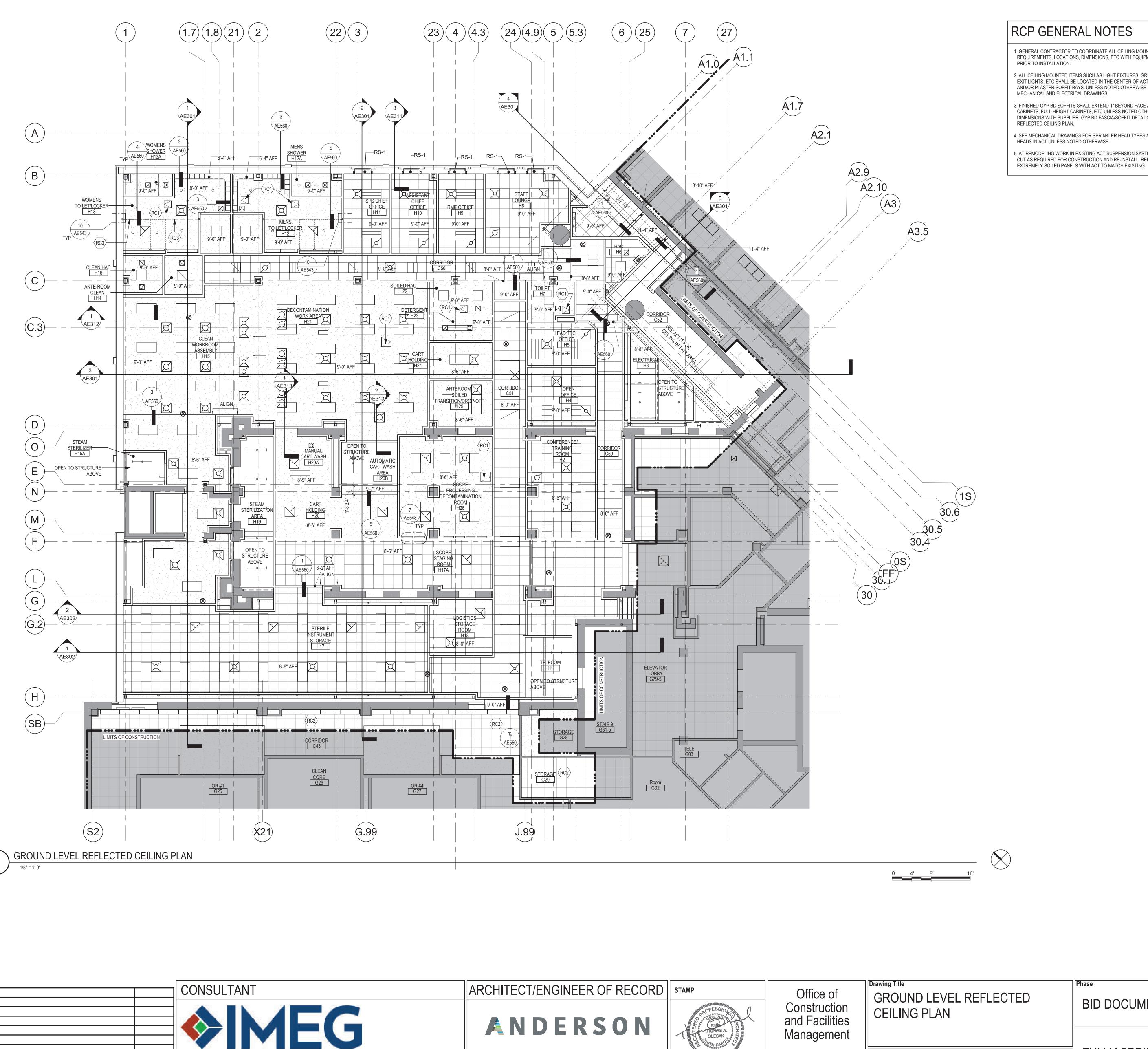
 10. PATCH AREAS DISTURBED OR MODIFIED BY THIS WORK WITH MATERIALS TO
- MATCH EXISTING ADJACENT SRUFACES OR SCHEDULED FINISHES AND TO A LIKE NEW CONDITION.
- 11. PROVIDE ACCESS PANELS WHERE INDICATED ON DRAWINGS AND WHERE DESCRIBED IN SPEFICATIONS SECTION 08 31 13. COORDINATE LOCATIONS AND SIZES REQUIRED (24" X 24" MINIMUM) WITH MECHANICAL, PLUMBING, ELECTRICAL AND TECHNOLOGY SUBCONTRACTORS.
- 12. ABREVIATIONS AND SYMBOLS OF ARCHITECTURAL, STRUCTURAL, MECHANICAL, AND ELECTRICAL MAY VARY BY DISCIPLINE. SEE RESPECTIVE DISIPLINE SECTIONS AND/OR DRAWINGS FOR DEFINITIONS.
- 13. AT MECHANICAL AND ELECTRICAL EQUIPMENT ROOMS, PROVIDE 3/4" FIRE RETARDANT TREATED (FRT) PLYWOODON SURFACE OF SCHEDULED PARTITION FOR SURFACE MOUNTED FIXTURES AND EQUIPMENT, UNLESS NOTED OTHERWISE. DO NOT PAINT OVER FRT LABEL.
- 14. EVALUATE INTEGRITY OF EXISTING LIFE SAFETY RATED PARTITION ASSEMBLIES INDICATED BY SYMBOLS ON THE LIFE SAFETY PLAN(S) WITHIN THE PROJECT SCOPE INCLUDING PENETRATIONS THOUGH ASSEMBLY AND BASE AND HEAD OF ASSEMBLY. WHERE GAPS OR DEFFICIES ARE FOUND, RESTORE INTEGRITY OF EXISTING ASSEMBLIES WITH MATERIAL NECESSARY TO MEET REQUIRED REATING OF THE ASSEMBLIES.



Project Title Project Number 438-460 CONSTRUCT NEW SPS BID DOCUMENTS **IMEG** Construction Building Number and Facilities ANDERSON Management Drawing Number Sioux Falls, SD. **FULLY SPRINKLERED** Checked 13605 1st Ave. N. #100 Plymouth, MN 55441 P 763.412.4000 | F 763.412.4090 | ae-mn.com U.S. Department of Veterans Affairs AE102 DATE: <u>08/04/2022</u> 08/04/2022 Checker Author Revisions: Anderson Engineering of Minnesota, LLC | Proj # 16584 License # 9335 VA FORM 08 - 6231







ANDERSON

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Anderson Engineering of Minnesota, LLC | Proj # 16584

Revisions:

VA FORM 08 - 6231

DATE: <u>08/04/2022</u>

License # 9335

Management

RCP GENERAL NOTES

1. GENERAL CONTRACTOR TO COORDINATE ALL CEILING MOUNTED EQUIPMENT SUPPORT REQUIREMENTS, LOCATIONS, DIMENSIONS, ETC WITH EQUIPMENT SUPPLIER AND OWNER, PRIOR TO INSTALLATION.

2. ALL CEILING MOUNTED ITEMS SUCH AS LIGHT FIXTURES, GRILLES, DIFFUSERS, SPEAKERS, EXIT LIGHTS, ETC SHALL BE LOCATED IN THE CENTER OF ACT PANELS, GYP BD SOFFITS AND/OR PLASTER SOFFIT BAYS, UNLESS NOTED OTHERWISE. COORDINATE WITH MECHANICAL AND ELECTRICAL DRAWINGS.

3. FINISHED GYP BD SOFFITS SHALL EXTEND 1" BEYOND FACE AND EXPOSED ENDS OF WALL CABINETS, FULL-HEIGHT CABINETS, ETC UNLESS NOTED OTHERWISE. COORDINATE CABINET DIMENSIONS WITH SUPPLIER. GYP BD FASCIA/SOFFIT DETAILS ARE REFERENCED FROM THE REFLECTED CEILING PLAN.

4. SEE MECHANICAL DRAWINGS FOR SPRINKLER HEAD TYPES AND LOCATIONS. CENTER ALL HEADS IN ACT UNLESS NOTED OTHERWISE.

5. AT REMODELING WORK IN EXISTING ACT SUSPENSION SYSTEM, REMOVE EXISTING PANELS, CUT AS REQUIRED FOR CONSTRUCTION AND RE-INSTALL. REPLACE ALL DAMAGED AND / OR

REFLECTED CEILING PLAN SYMBOLS							
	GYP BD CEILING / SOFFIT	OR	2'-0" x 4'-0" LIGHT FIXTURE				
	ACT CEILING SYSTEM		2'-0" x 2'-0" LIGHT FIXTURE				
	EXISTING ACT CEILING SYSTEM	<u></u>	PENDANT STRIP LIGHT FIXTURE				
	IV TRACK	0 0 0	PENDANT STRIP LIGHT FIXTURE				
	CUBICLE CURTAIN TRACK	0	RECESSED LIGHT FIXTURE				
	ACCESS PANEL	7	WALL SCONCE LIGHT FIXTURE				
	MECHANICAL SUPPLY GRILL		MECHANICAL LINEAR SUPPLY GRILL				
Ø	MECHANICAL RETURN GRILL	9'-0" AFF	CEILING ELEVATION				

REFLECTED CEILING KEYED NOTES

18' X 18" STAINLESS STEEL ACCESS PANEL. COORDINATE LOCATION WITH MECHANICAL, ELECTRICAL AND PLUMBING AND PER SEPC SECTION 08 31

NOTE: NOT ALL SYMBOLS MAY BE USED ON EACH PLAN

REMOVE AND REPLACE CEILING, INCLUDING ANY MECHANICAL / RC2 ELECTRICAL CEILING MOUNTED ITEMS, IN THIS AREA AS REQUIRED FOR MECHANICAL WORK. REPLACE ANY DIRTY OR DAMAGED ACT OR CEILING GRID. SEE MEP PLANS FOR ADDITIONAL INFORMATION

RC3 STEEL CHANNEL AND STEEL POST. SEE DETAILS

NOTE: NOT ALL KEYED NOTES MAY BE USED ON EACH PLAN

Project Title Project Number 438-460 CONSTRUCT NEW SPS BID DOCUMENTS Building Number **CEILING PLAN** Drawing Number Sioux Falls, SD. FULLY SPRINKLERED | Checked | Drawn U.S. Department of Veterans Affairs AC101 08/04/2022 Checker Author